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12 INDIAN WELLS VALLEY WATER DISTRICT

13 SUPERIOR COURT OF THE STATE OF CALIFORNIA
14 FOR THE COUNTY OF ORANGE, CIVIL COMPLEX CENTER
15

16 MOJAVE PISTACHIOS, LLC; et al.,

17 Plaintiffs,

18 v.

19 INDIAN WELLS VALLEY WATER
20 DISTRICT; et al.,

21 Defendants.
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Case No. 30-2021-01187275-CU-OR-CJC

**NOTICE OF MOTION & MOTION OF
INDIAN WELLS VALLEY WATER
DISTRICT FOR ORDER TO:**

- (1) DIVIDE TRIAL INTO PHASES;**
- (2) ISSUE AN ORDER TO SHOW CAUSE
RE: BASIN BOUNDARY;**
- (3) SET A PHASE 1 TRIAL RE:
GROUNDWATER IN STORAGE &
UNITED STATES' FEDERAL
RESERVED WATER RIGHT CLAIM; &**
- (4) PARTIALLY LIFT DISCOVERY STAY;
MEMORANDUM OF POINTS &
AUTHORITIES; & DECLARATIONS OF
DOUGLAS J. EVERTZ & TIMOTHY K.
PARKER, PG, CEG, CHG**

Date: March 22, 2024
Time: 1:30 p.m.
Dept.: CX101
Reservation No. 74172981

{00271736.2 }

1 INDIAN WELLS VALLEY WATER
2 DISTRICT,

3 Cross-Complainant,

4 v.

5 ALL PERSONS WHO CLAIM A
6 RIGHT TO EXTRACT
7 GROUNDWATER IN THE INDIAN
8 WELLS VALLEY GROUNDWATER
9 BASIN NO. 6-54 WHETHER BASED
10 ON APPROPRIATION, OVERLYING
11 RIGHT, OR OTHER BASIS OF
12 RIGHT, AND/OR WHO CLAIM A
13 RIGHT TO USE OF STORAGE SPACE
14 IN THE BASIN; et al.,

15 Cross-Defendants.

16 SEARLES VALLEY MINERALS INC.,

17 Cross-Complainant,

18 v.

19 ALL PERSONS WHO CLAIM A
20 RIGHT TO EXTRACT
21 GROUNDWATER IN THE INDIAN
22 WELLS VALLEY GROUNDWATER
23 BASIN NO. 6-54 WHETHER BASED
24 ON APPROPRIATION, OVERLYING
25 RIGHT, OR OTHER BASIS OF
26 RIGHT, AND/OR WHO CLAIM A
27 RIGHT TO USE OF STORAGE SPACE
28 IN THE BASIN; et al.,

Cross-Defendants.

AND RELATED CASES.

[Related to: Case No. 30-2021-01187589-CU-WM-CXC; Case No. 30-2021-01188089-CU-WM-CXC; Case No. 30-2022-01239479-CU-MC-CJC; Case No. 30-2022-01239487-CU-MC-CJC; Case No. 30-2022-01249146-CU-MC-CJC]

Assigned For All Purposes To:
The Honorable William Claster, Dept. CX101

Complaint Filed: November 19, 2019

Trial Date: None Set

1 **NOTICE OF MOTION**

2 **TO ALL PARTIES AND TO THEIR ATTORNEYS OF RECORD:**

3 **PLEASE TAKE NOTICE** that on March 22, 2024, at 1:30 p.m., or as soon thereafter as
4 the matter may be heard, in Department CX101 of the Orange County Superior Court - Civil
5 Complex Center, located at 751 West Santa Ana Boulevard, Santa Ana, California 92701,
6 Defendant, Cross-Complainant, and Cross-Defendant Indian Wells Valley Water District
7 (“District”) will and hereby does move (“Motion”) for an order:

8 (1) **Phases of Trial:** Dividing trial of this comprehensive groundwater basin
9 adjudication into phases;

10 (2) **OSC re Basin Boundary:** Issuing an Order to Show Cause directing that
11 the Indian Wells Valley Groundwater Basin boundary as currently determined by the California
12 Department of Water Resources (DWR Basin No. 6-54) (“Basin”) is the groundwater basin
13 boundary for purposes of this comprehensive adjudication and requiring any party that contends
14 otherwise to immediately show cause why the Basin boundary should not be in accordance with
15 DWR Basin No. 6-54, including supporting evidence briefing.

16 (3) **Phase 1 Trial:** Setting a phase 1 bench trial (“Phase 1 Trial”) and
17 defining the scope of issues to be tried at the Phase 1 Trial to consist of determining the amount
18 of groundwater in storage within the Basin, including the amount of available fresh water in
19 storage; and adjudicating the federal reserved water right claim of Cross-Defendant United States
20 of America (“United States”);

21 (4) **Discovery:** Lifting the stay on discovery, but only as to the issues to be
22 tried during the Phase 1 Trial; and

23 (5) **Expert Disclosures:** Lifting the stay on expert witness disclosures and
24 setting a deadline for the exchange of expert witness disclosures, but only as to the issues to be
25 tried during the Phase 1 Trial.

26 Looking ahead, District expects to file another motion for order setting a phase 2 bench
27 trial (“Phase 2 Trial”) to occur expeditiously after the Phase 1 Trial. The anticipated scope of the
28 Phase 2 Trial will consist of determining the safe yield of the Basin; adjudicating all water rights,

1 such as the nature of the rights and their relative priority; and considering and adopting a
2 physical solution, all consistent with the Court’s findings in the Phase 1 Trial.

3 District makes this Motion under Code of Civil Procedure sections 598, 840(b)(5),
4 1048(b), and 2019.020(b); and the Court’s inherent authority to control the matters before it.
5 (Code Civ. Proc., §§ 128(a)(8), 187; Govt. Code, § 68070(a); *Cottle v. Super. Ct.* (1992) 3
6 Cal.App.4th 1367, 1376-1379.) District makes the Motion on the following grounds:

7 A. The Court has express statutory authority to divide trial of this comprehensive
8 adjudication into phases and to phase discovery according to the phases of trial.

9 B. Conducting the trial of this comprehensive adjudication in phases will promote
10 judicial efficiency, the convenience of the Court and parties, and the ends of justice.

11 C. California law vests courts with exclusive authority to determine groundwater
12 rights—authority expressly not held by groundwater sustainability agencies, the California
13 Department of Water Resources, or the California State Water Resources Control Board.

14 D. This Court has both the constitutional duty to consider a physical solution
15 proposed by the parties and the authority to impose the physical solution over objection by one
16 or more parties in a comprehensive adjudication.

17 E. A court considers the adoption of a physical solution to optimize the beneficial
18 use of the available groundwater in accordance with California Constitution Article X, section 2
19 and to sustainably manage the groundwater basin in accordance with the Sustainable
20 Groundwater Management Act of 2014 (Water Code sections 10720 to 10738).

21 F. Essential components in determining water rights and a fair, equitable, and
22 workable physical solution are the amount of groundwater in storage within the Basin and the
23 Basin’s safe yield.

24 G. To determine storage capacity, quantity of groundwater in storage, and safe yield,
25 courts must consider technical data and expert opinion.


26 H. Upon entry of a final judgment and physical solution, the court retains jurisdiction
27 to address changes in circumstances including, but not limited to, changes in hydrologic
28

1 conditions; to administer the physical solution; and to resolve future disputes among the parties
2 during administration of the physical solution.

3 The Motion is based upon this Notice of Motion and Motion, the accompanying
4 Memorandum of Points and Authorities, and the accompanying Declarations of Douglas J.
5 Evertz and Timothy K. Parker, PG, CEG, CHG; all other pleadings and papers on file in this
6 action; all matters of which the Court may take judicial notice; and any further evidence or oral
7 argument presented to the Court before or during the hearing on this Motion.

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DATED: February 23, 2024 MURPHY & EVERTZ LLP

By: 

Douglas J. Evertz
Emily L. Madueno
Attorneys for Defendant, Cross-Complainant, &
Cross-Defendant
INDIAN WELLS VALLEY WATER DISTRICT

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23
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27
28

TABLE OF CONTENTS

Page

1. INTRODUCTION 1

2. BACKGROUND 2

 A. SGMA 2

 B. The Streamlined Act 3

 C. Only a Court—Not DWR, the State Board, or any Groundwater
Sustainability Agency—has the Power to Determine Groundwater
Rights. 4

 D. The Technical Working Group 5

3. CALIFORNIA LAW EMPOWERS THIS COURT TO DIVIDE TRIAL
OF A COMPREHENSIVE ADJUDICATION INTO PHASES AND TO
PHASE DISCOVERY ACCORDINGLY. 6

4. GOOD CAUSE EXISTS TO DIVIDE TRIAL OF THIS
COMPREHENSIVE ADJUDICATION INTO PHASES AND TO
SCHEDULE DISCOVERY CONSISTENT WITH THE PHASING OF
TRIAL. 7

5. COURTS HAVE EXCLUSIVE AUTHORITY TO DETERMINE WATER
RIGHTS AND TO IMPOSE PHYSICAL SOLUTIONS. 9

6. THE COURT’S FINDINGS ON STORAGE ARE KEY TO
DETERMINING SAFE YIELD AND ULTIMATELY ADOPTING A
PHYSICAL SOLUTION TO ACHIEVE SUSTAINABLE
GROUNDWATER MANAGEMENT. 10

 A. Courts Determine Storage and Safe Yield and Retain Jurisdiction to
Redetermine Them as Conditions Change. 10

 B. Trial Courts Must Consider Technical Data and Expert Testimony
to Determine Storage and Safe Yield. 11

 C. The Jurisdiction of Trial Courts to Consider Technical Data
Through Adjudication Concurrent with Groundwater Sustainability
Agencies, DWR, and the State Board Promotes Sustainable
Groundwater Management. 14

7. CONCLUSION 15

TABLE OF AUTHORITIES

Page(s)

CASES

Allen v. California Water & Telephone Co.
(1946) 29 Cal.2d 46611

California American Water v. City of Seaside (Seaside)
(2010) 183 Cal.App.4th 4714, 11, 13, 14

*Central & West Basin Water Replenishment District v. Southern California
Water Co.*
(2003) 109 Cal.App.4th 89111

City of Barstow v. Mojave Water Agency (Barstow)
(2000) 23 Cal.4th 12245, 10

City of Lodi v. East Bay Municipal Utility District (Lodi)
(1936) 7 Cal.2d 3165, 10

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(1975) 14 Cal.3d 1998, 11, 13, 14

City of Pasadena v. City of Alhambra (Pasadena)
(1949) 33 Cal.2d 9088, 11, 14, 15

City of Santa Maria v. Adam (Santa Maria I)
(2012) 211 Cal.App.4th 2664, 5, 8, 10

Cottle v. Superior Court
(1992) 3 Cal.App.4th 13677

Daubert v. Merrell Dow Pharmaceuticals, Inc.
(1993) 509 U.S. 579.....15

Environmental Defense Fund v. East Bay Municipal Utility District
(1980) 26 Cal.3d 18313

Hillside Memorial Park & Mortuary v. Golden State Water Co. (Hillside)
(2011) 205 Cal.App.4th 5344, 9, 10, 11, 14

International Association of Fire Fighters v. Public Employment Relations Board
(2011) 51 Cal.4th 25912

Peabody v. City of Vallejo (Peabody)
(1935) 2 Cal.2d 3515, 10

People ex rel. Brown v. Tri-Union Seafoods, LLC
(2009) 171 Cal.App.4th 154915

1	<i>Phelan Piñon Hills Community Services District v. California Water Service Co.</i>	
2	(<i>Antelope Valley Groundwater Cases</i>) (<i>Antelope Valley I</i>)	
	(2020) 59 Cal.App.5th 241	6, 11
3	<i>Rancho Santa Margarita v. Vail</i> (<i>Rancho Santa Margarita</i>)	
4	(1938) 11 Cal.2d 501	5, 10
5	<i>Tulare Irrigation District v. Lindsay-Strathmore Irrigation District</i>	
6	(1935) 3 Cal.2d 489	10
7	<i>Willis v. Los Angeles County Waterworks District No. 40</i> (<i>Antelope Valley</i>	
8	<i>Groundwater Cases</i>) (<i>Antelope Valley II</i>)	
	(2021) 62 Cal.App.5th 992	4, 8, 11, 13, 14
9	<i>Wright v. Goleta Water District</i> (<i>Wright</i>)	
10	(1985) 174 Cal.App.3d 74	4, 9, 12
11		
12	FEDERAL STATUTES	
13	43 U.S.C. section 666, McCarran Amendment	8
14		
15	STATE CONSTITUTIONAL PROVISIONS	
16	California Constitution, Article X, Section 2.....	5, 10, 13, 15
17		
18	STATE STATUTES	
19	California Code of Civil Procedure:	
20	Section 128(a)(8)	6
	Section 187.....	6
21	Section 598.....	6
22	Sections 830-852, California Streamlined Groundwater Adjudication Act	
	of 2015 (Streamlined Act)	2
	Section 830.....	4
23	Section 830(b)(7)	13, 15
24	Section 832(c)	4
	Section 834.....	8
25	Section 834(a)	4
	Section 834(b).....	4, 9
26	Section 836(a)(2)(B)	7
	Section 838(b).....	8
27	Section 840(b)(5)	6
	Section 840(b)(7)	6
28	Section 845(a)(1)	12

1	Section 845(a)(2)	12
	Section 848(a)(2)	12
2	Section 849(a)	5, 8, 10
3	Section 849(b)	12
	Section 852	11
4	Section 1048(b)	6
	Section 2019.020(b)	7
5		
6	California Government Code Section 68070(a)	7
7	California Water Code:	
	Section 10720-10738, Sustainable Groundwater Management Act of 2014	
8	(SGMA)	2
	Section 10720.1(a)	2
9	Section 10720.1(b)	5, 9
	Section 10720.5	13
10	Section 10720.5(a)	15
	Section 10720.5(b)	4, 9
11	Section 10720.5(c)	4, 5, 9
	Section 10720.7(a)	3
12	Section 10721(a)	4
	Section 10721(n)	12
13	Section 10721(v)	14
14	Section 10721(w)	14
	Section 10726.8(b)	4, 9
15	Section 10727(a)	3
16	Section 10733	3
	Section 10733.4	3
17	Section 10733.6(a)	12
	Section 10733.6(b)(2)	12
18	Section 10737	3, 9
	Section 10737.2	14, 15
19	Section 10737.4	12
	Section 10737.4(b)	12
20	Section 10737.4(c)	12
	Section 10737.6	12
21	Section 10738	3, 10
22		

23		
24	CALIFORNIA LEGISLATIVE MATERIALS	
25	Assembly Committee on Water, Parks, and Wildlife, Report on Assembly Bill	
	No. 1390 (2015-2016 Reg. Sess.) Apr. 14, 2015	12, 14
26		
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	of Senate Bill No. 1372 (2021-2022 Reg. Sess.) as amended Mar. 16, 2022	3
28		

1 **MEMORANDUM OF POINTS AND AUTHORITIES**

2 **1. INTRODUCTION**

3 On June 16, 2021, the Indian Wells Valley Water District (“District”) initiated a
4 comprehensive adjudication of the Indian Wells Valley Groundwater Basin (“Basin”), pursuant
5 to the California Streamlined Groundwater Adjudication Act, when it filed a Cross-Complaint
6 for Comprehensive Adjudication (“Comprehensive Adjudication”). District seeks a judgment in
7 this case that will comprehensively adjudicate all rights to extract and store groundwater in the
8 Basin and establish a physical solution to maximize the reasonable and beneficial use of
9 groundwater while sustainably managing the Basin under the Court’s continuing jurisdiction.

10 Under California law, a physical solution is an often agreed-upon or judicially imposed
11 resolution of conflicting claims to water that advances the constitutional rule of maximizing the
12 reasonable and beneficial uses of the state’s water supply without causing undesirable results and
13 while respecting established water rights. This Court has not only the constitutional authority,
14 but also the constitutional duty, to consider and impose a physical solution on the parties to this
15 Comprehensive Adjudication.

16 District’s motion (“Motion”) offers the Court a case roadmap—designed to promote
17 judicial efficiency, the convenience of the Court and the parties, and the ends of justice—to
18 assist the Court in fulfilling its constitutional duty to impose a physical solution. District seeks
19 an order to:

20 A. **Trial Phasing**: Divide trial of the Comprehensive Adjudication into phases;
21 B. **OSC re Basin Boundary**: Issue an Order to Show Cause directing that the Basin
22 boundary as currently determined by the California Department of Water Resources (“DWR”)
23 (DWR Basin No. 6-54) is the groundwater basin boundary for purposes of this Comprehensive
24 Adjudication and requiring any party that contends otherwise to immediately show cause why
25 the Basin boundary should not be in accordance with DWR Basin No. 6-54, including supporting
26 evidence briefing.

27 C. **Phase 1 Trial**: Set a phase 1 bench trial (“Phase 1 Trial”) and define the scope of
28 issues to be tried at the Phase 1 Trial to consist of determining the amount of groundwater in

1 storage within the Basin, including the amount of available fresh water in storage; and
2 adjudicating the federal reserved water right claim of Cross-Defendant United States of America
3 (“United States”);

4 D. **Discovery**: Lift the stay on discovery, but only as to the issues to be tried during
5 the Phase 1 Trial; and

6 E. **Expert Witness Disclosures**: Lift the stay on expert witness disclosures and set
7 a deadline for their exchange, but only as to the issues to be tried during the Phase 1 Trial.

8 District seeks a first phase trial on the amount of groundwater in storage and the United
9 States’ federal reserved water right claim because both are key findings needed for the ultimate
10 physical solution. For example, the best-estimate of the amount of fresh groundwater in storage
11 of the Technical Working Group—a minimum of 37.5 million Acre Feet (“AF”)—is more than
12 21 times that in the Basin’s current Groundwater Sustainability Plan (“GSP”)—1.75 million AF.
13 Moreover, the GSP adopted by the Indian Wells Valley Groundwater Authority (“IWVGA”)
14 currently allocates the entire estimated “sustainable yield” (as defined by IWVGA) of the Basin
15 to the United States, making the Court’s finding on the federal reserved water right claim a
16 significant element of the physical solution.

17 Looking ahead, District expects to file another motion for an order setting a phase 2
18 bench trial (“Phase 2 Trial”) to occur expeditiously after the Phase 1 Trial and to determine the
19 Basin’s safe yield, to adjudicate all water rights, and to consider and adopt a physical solution,
20 all consistent with the Court’s findings in the Phase 1 Trial.

21 **2. BACKGROUND**

22 This case is one of the first applications of the interplay between the Sustainable
23 Groundwater Management Act of 2014 (Water Code sections 10720 to 10738, “SGMA”) and the
24 California Streamlined Groundwater Adjudication Act of 2015 (Code of Civil Procedure
25 sections 830 to 852, “Streamlined Act”).

26 **A. SGMA**

27 In 2014, California enacted SGMA to promote sustainable management of groundwater
28 basins throughout the state. (Wat. Code, § 10720.1(a).) To that end, SGMA requires local

1 groundwater sustainability agencies to manage medium- and high-priority groundwater basins—
2 as designated by DWR—through groundwater sustainability plans with oversight from DWR
3 and the State Water Resources Control Board (“State Board”). (See Wat Code., § 10720.7(a).)
4 Under SGMA, groundwater sustainability agencies in DWR-designated “critically overdrafted”
5 basins of medium- and high-priority were required to adopt groundwater sustainability plans by
6 January 31, 2020. (Wat. Code, §§ 10720.7(a), 10727(a).) Once a groundwater sustainability
7 agency adopts a groundwater sustainability plan for its basin, the agency then must submit its
8 plan to DWR for review and assessment. (Wat. Code, §§ 10733, 10733.4.)

9 Here, DWR designated the Basin as a critically overdrafted groundwater basin of high
10 priority. On January 16, 2020, the Basin’s groundwater sustainability agency—IWVGA—
11 adopted a groundwater sustainability plan for the Basin (“GSP”). DWR approved the GSP on
12 January 13, 2022, but found that the GSP “does not fully satisfy . . . the requirements of
13 [DWR’s] GSP Regulations” and identified numerous “corrective actions.” DWR also refused to
14 endorse the legal adequacy of the GSP and the water rights determinations therein, which
15 determinations are the province of the Court. (See Declaration of Timothy K. Parker, PG, CEG,
16 CHG (“Parker Decl.”), ¶ 6.)

17 Further, SGMA confirms that DWR’s approval of the GSP “shall not be construed to be a
18 determination by or otherwise an opinion of [DWR] that the allocation of groundwater pumping
19 rights in the [GSP] are [sic] consistent with groundwater rights law.” (Wat. Code, § 10738
20 [emphasis added].) In fact, Water Code section 10738 was enacted as a result of concerns over
21 the GSP in this particular Basin to clarify that DWR’s approval of the GSP should not be
22 “interpreted by the courts to be an endorsement of the allocation of pumping rights as embodied
23 in the GSP.” (Sen. Rules Com., Off. of Sen. Floor Analyses, 2d reading analysis of Sen. Bill
24 No. 1372 (2021-2022 Reg. Sess.) as amended Mar. 16, 2022, p. 2.) Instead, SGMA confirms
25 that determining groundwater rights must occur through an adjudication, which “shall be
26 conducted in accordance with [the Streamlined Act].” (Wat. Code, § 10737.)

27 **B. The Streamlined Act**

28 In 2015, California enacted the Streamlined Act to streamline procedures for

1 comprehensive groundwater adjudications to reduce the time and cost incurred to determine
2 groundwater rights. (See Code Civ. Proc., § 830.) A comprehensive adjudication is the only
3 way to comprehensively determine all water rights in a groundwater basin. (Code Civ. Proc.,
4 § 834(a); *Willis v. L.A. County Waterworks Dist. No. 40 (Antelope Valley Groundwater Cases)*
5 (2021) 62 Cal.App.5th 992, 1025-1026, 1035 (*Antelope Valley II*); *Hillside Memorial Park &*
6 *Mortuary v. Golden State Water Co.* (2011) 205 Cal.App.4th 534, 549 (*Hillside*); cf. Wat. Code,
7 § 10720.5(b) & (c); *Wright v. Goleta Water Dist.* (1985) 174 Cal.App.3d 74, 88-89 (*Wright*.)
8 Indeed, SGMA and the Streamlined Act define an “adjudication” as “an action filed in superior
9 court to comprehensively determine rights to extract groundwater in a basin.” (Code Civ. Proc.,
10 § 832(c); Wat. Code, § 10721(a).)

11 “The court’s final judgment in a comprehensive adjudication, for the groundwater rights
12 of each party, may declare the priority, amount, purpose of use, extraction location, place of use
13 of the water, and use of storage space in the basin . . . subject to terms adopted by the court to
14 implement a physical solution in the comprehensive adjudication.” (Code Civ. Proc., § 834(b).)
15 “The phrase ‘physical solution’ is used in water-rights cases to describe an agreed upon or
16 judicially imposed resolution of conflicting claims in a manner that advances the constitutional
17 rule of reasonable and beneficial use of the state’s water supply.” (*City of Santa Maria v. Adam*
18 (2012) 211 Cal.App.4th 266, 287-288 (*Santa Maria I*); *Cal. American Water v. City of Seaside*
19 (2010) 183 Cal.App.4th 471, 480 (*Seaside*).)

20 **C. Only a Court—Not DWR, the State Board, or any Groundwater**
21 **Sustainability Agency—has the Power to Determine Groundwater Rights.**

22 The authority to determine groundwater rights is reserved exclusively to the courts.
23 (*Hillside, supra*, 205 Cal.App.4th at 549; *Wright, supra*, 174 Cal.App.3d at 87 [confirming
24 groundwater exempted from statutory adjudication procedure for surface water].) “Nothing in
25 [SGMA], or in any groundwater management plan adopted pursuant to [SGMA], determines or
26 alters surface water rights or groundwater rights under common law or any provision of law that
27 determines or grants surface water rights.” (Wat. Code, § 10720.5(b); see also Wat. Code,
28 § 10726.8(b) [nothing in SGMA authorizes “a local agency to make a binding determination of

1 the water rights of any person or entity”).) SGMA confirms that “[w]ater rights may be
2 determined in an adjudication action pursuant to [the Streamlined Act].” (Wat. Code,
3 § 10720.5(c); see also Wat. Code, § 10720.1(b).) Courts alone “have the authority and the duty
4 to impose a physical solution on the parties in a comprehensive adjudication where necessary
5 and consistent with Article 2 [sic] of Section X [sic] of the California Constitution.” (Code Civ.
6 Proc., § 849(a); e.g., *City of Barstow v. Mojave Water Agency* (2000) 23 Cal.4th 1224, 1250
7 (*Barstow*) [citing *City of Lodi v. East Bay Mun. Util. Dist.* (1936) 7 Cal.2d 316, 341 (*Lodi*);
8 *Rancho Santa Margarita v. Vail* (1938) 11 Cal.2d 501, 558-559 (*Rancho Santa Margarita*);
9 *Peabody v. City of Vallejo* (1935) 2 Cal.2d 351, 367-368, 383-384 (*Peabody*); *Santa Maria I*,
10 *supra*, 211 Cal.App.4th at 288.)

11 **D. The Technical Working Group**

12 District and several other parties—who altogether accounted for more than 80 percent of
13 the Basin’s total groundwater production in water year 2022¹—organized a group of qualified
14 groundwater professionals (“Technical Working Group”). (Parker Decl., ¶ 7.) The Technical
15 Working Group Parties’ technical consultants are among the most well-respected groundwater
16 professionals in the state, including Krieger & Stewart, Parker Groundwater, Ramboll, Luhdorff
17 & Scalmanini, Aquilogic, and Geoscience (*Id.* at ¶¶ 8-13.) These consultants have been
18 involved in many prior adjudications. (See *id.* at ¶¶ 10-13.)

19 The Technical Working Group met regularly over the last year and a half. (*Id.* at ¶ 7.) It
20 analyzed all available data relating to the Basin and Basin groundwater, including IWVGA’s
21 GSP; performed additional analyses; and relied on the best available science. (See *id.* at ¶ 14.)
22 They collaboratively evaluated the Basin’s size, characteristics, and capacity to develop a
23 best-estimate of the amount of cumulative groundwater in storage. The Technical Working
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25 ¹ (1) Plaintiffs and Cross-Defendants Mojave Pistachios, LLC; John Thomas Conaway; John Thomas Conaway
26 Trust; John Thomas Conaway Living Trust u/d/t August 7, 2008; Nugent Family Trust; and Sierra Shadows Ranch
27 LP (collectively, “Mojave Pistachios”); (2) District; (3) Defendant, Cross-Defendant, and Cross-Complainant
28 Searles Valley Minerals (“Searles”); and (4) Defendants and Cross-Defendants Meadowbrook Dairy Real Estate,
LLC; Big Horn Fields, LLC; Brown Road Fields, LLC; Highway 395 Fields, LLC; and the Meadowbrook Mutual
Water Company (collectively, “Meadowbrook”) (altogether, “Technical Working Group Parties”) represent all
major Basin pumpers, except the United States, though it was invited to participate. (Evertz Decl., ¶ 4.)

1 Group is currently documenting a best-estimate of the Basin’s safe yield and developing
2 potential basin management strategies to sustainably manage the Basin consistent with SGMA
3 and a physical solution to maximize beneficial use of Basin groundwater consistent with the
4 California Constitution, while respecting water rights without causing undesirable results. (See
5 *id.* at ¶¶ 14-19.) The Technical Working Group Parties are prepared to present their findings on
6 Basin groundwater in storage to the Court in the Phase 1 Trial. (See *id.* at ¶ 15, Exh. A [Storage
7 Paper].)

8 **3. CALIFORNIA LAW EMPOWERS THIS COURT TO DIVIDE TRIAL OF A**
9 **COMPREHENSIVE ADJUDICATION INTO PHASES AND TO PHASE**
10 **DISCOVERY ACCORDINGLY.**

11 The Court possesses express statutory authority to divide trial of this Comprehensive
12 Adjudication into phases on discrete issues and to phase discovery accordingly.

13 The Streamlined Act empowers a court to “[d]ivid[e] the case into phases to resolve legal
14 and factual issues.” (Code Civ. Proc., § 840(b)(5).) Likewise, case law recognizes that in an
15 adjudication, “[a] trial court has discretion to determine the order in which claims or issues are
16 bifurcated and determined, and the selection and scheduling of those phased determinations will
17 not be disturbed absent an abuse of that discretion.” (*Phelan Piñon Hills Cmty. Servs. Dist. v.*
18 *Cal. Water Service Co. (Antelope Valley Groundwater Cases)* (2020) 59 Cal.App.5th 241, 273
19 (*Antelope Valley I*) [affirming court’s discretion to determine the order of the phases of trial on
20 discrete issues in a water rights adjudication].) The Streamlined Act also empowers courts to
21 “limit[] discovery to correspond to the phases” of trial. (Code Civ. Proc., § 840(b)(7).)

22 Consistent with and complementary to the Streamlined Act, general civil litigation
23 sections of the Code of Civil Procedure authorize and encourage bifurcation and separate trials of
24 issues to facilitate judicial efficiency in handling litigation and to promote the ends of justice.
25 (Code Civ. Proc., § 598 [authorizing phasing of trial]; Code Civ. Proc., § 1048(b) [authorizing
26 bifurcation of trial]; see also Code Civ. Proc., § 128(a)(8) [“Every court shall have the power to .
27 . . . control its process and orders so as to make them conform to law and justice.”]; Code Civ.
28 Proc., § 187 [“any suitable process or mode of proceeding may be adopted which may appear

1 most conformable to the spirit of this Code”]; Govt. Code, § 68070(a) [“Every court may make
2 rules for its own government . . . not inconsistent with law or with the rules adopted and
3 prescribed by the Judicial Council.”]; *Cottle v. Super. Ct.* (1992) 3 Cal.App.4th 1367, 1376-1379
4 [courts have “inherent power to control litigation before them”].) The general civil litigation
5 Code sections also empower courts to “establish the sequence and timing of discovery for the
6 convenience of the parties and witnesses and in the interests of justice,” similar to the
7 Streamlined Act. (Code Civ. Proc., § 2019.020(b).)

8 **4. GOOD CAUSE EXISTS TO DIVIDE TRIAL OF THIS COMPREHENSIVE**
9 **ADJUDICATION INTO PHASES AND TO SCHEDULE DISCOVERY**
10 **CONSISTENT WITH THE PHASING OF TRIAL.**

11 Dividing trial of this Comprehensive Adjudication—a substantively complex case that
12 requires the Court to determine all rights to extract and store groundwater within the Basin—into
13 phases and scheduling discovery consistent with those phases will promote judicial efficiency,
14 the convenience of the Court and the parties, and the ends of justice for multiple reasons.

15 First, basin adjudications raise several substantive issues that a court can manage most
16 efficiently and effectively in sequence to arrive at a final judgment imposing a physical solution.
17 Namely, District recommends that the Court address the issues in the following order:

- 18 • Order to Show Cause re Basin Boundary: SGMA and the Streamlined Act
19 presume the Basin boundary is as defined in DWR’s Bulletin 118 Report. (Code
20 Civ. Proc., §§ 832(a), 841; Wat. Code, §§ 10721(b), 10722.) Neither IWVGA nor
21 any party sought to change the Basin boundary through the DWR Basin Boundary
22 Modification processes in 2016 or 2018 or alleged in its answer its intention to
23 seek adjustment of the Basin’s boundary under Code of Civil Procedure
24 section 836(a)(2)(B). To preserve valuable party and judicial resources, the Court
25 should immediately issue an Order to Show Cause to confirm that the Basin
26 boundary will be as set forth by the current Bulletin 118 Report.
- 27 • Phase 1 Trial: Trial to (a) determine the amount of Basin groundwater in storage,
28 including the amount of available fresh water in storage; and (b) adjudicate the

1 federal reserved water right claim.

- 2 • Phase 2 Trial: Trial to (c) determine the Basin’s safe yield, which is “the
3 maximum quantity of water which can be withdrawn annually from a ground
4 water supply under a given set of conditions without causing an undesirable
5 result” (*City of Los Angeles v. City of San Fernando* (1975) 14 Cal.3d 199, 278
6 (*San Fernando*)); (d) adjudicate all groundwater rights in the Basin, such as the
7 nature of the rights and their relative priority, including overlying, appropriative,
8 and any prescriptive rights (Code Civ. Proc., § 834); and (e) consider and adopt a
9 physical solution to manage the Basin (Code Civ. Proc., § 849(a)), all consistent
10 with the Court’s findings in the Phase 1 Trial.

11 Sequencing trial of issues in a comprehensive adjudication is essential to achieve an
12 effective final judgment. (See, e.g., *Antelope Valley II, supra*, 62 Cal.App.5th at 1001; *Santa*
13 *Maria I, supra*, 211 Cal.App.4th at 282.) District seeks a pretrial order to confirm the Basin’s
14 boundaries, and a first phase trial to determine the amount of groundwater in storage and to
15 adjudicate the federal reserved water right claim because all are key findings needed for the
16 ultimate physical solution. For example, the best-estimate of the amount of fresh groundwater in
17 storage of the Technical Working Group—a minimum of 37.5 million AF—is more than 21
18 times that in the Basin’s current GSP—1.75 million AF. (Parker Decl., ¶ 18.) Moreover,
19 notwithstanding the United States’ written statement to incorporate an assumed 2,041 AFY
20 annual demand in the GSP, the GSP adopted by IWVGA allocates the entire “sustainable yield”
21 (as defined by IWVGA) of the Basin to the United States based upon water right presumptions,
22 making the Court’s finding on the federal reserved water right claim a significant element of the
23 ultimate physical solution. The McCarran Amendment (43 U.S.C. § 666) also requires that the
24 Court adjudicate the federal reserved water right claim.

25 Second, comprehensive adjudications are presumed to be complex cases. (Code Civ.
26 Proc., § 838(b); see *City of Pasadena v. City of Alhambra* (1949) 33 Cal.2d 908, 917 (*Pasadena*)
27 [recognizing “the complexity of the factual issues in water cases”]; *Antelope Valley II, supra*, 62
28 Cal.App.5th at 999 [recognizing the “legal and technical complexities inherent” in water rights

1 adjudications].) Like other complex civil litigation, comprehensive adjudications necessarily
2 involve highly technical data and expert testimony.

3 Generally, parties and the Court can better manage highly technical data in phases, rather
4 than all at once. The same is true here. In the first phase, the parties can focus on determining
5 how much water is in storage and adjudicating the federal reserved water right claim. Consistent
6 with the Court’s findings in phase one, in the second phase, the parties can focus on determining
7 the safe yield for the Basin, adjudicating all water rights, and presenting a fair, equitable, and
8 workable physical solution that implements the constitutional mandate to maximize the
9 reasonable beneficial use of water and the SGMA mandate to sustainably manage the Basin.

10 **5. COURTS HAVE EXCLUSIVE AUTHORITY TO DETERMINE WATER**
11 **RIGHTS AND TO IMPOSE PHYSICAL SOLUTIONS.**

12 California law vests courts with sole authority to determine groundwater rights.
13 (*Hillside, supra*, 205 Cal.App.4th at 549; *Wright, supra*, 174 Cal.App.3d at 87 [confirming
14 groundwater exempted from statutory adjudication procedure for surface water].) Indeed,
15 SGMA confirms that “[w]ater rights may be determined in an adjudication action pursuant to
16 [the Streamlined Act].” (Wat. Code, § 10720.5(c); see also Wat. Code, §§ 10720.1(b), 10737.)
17 The Streamlined Act provides, “The court’s final judgment in a comprehensive adjudication, for
18 the groundwater rights of each party, may declare the priority, amount, purposes of use,
19 extraction location, place of use of the water, and use of storage space in the basin” (Code
20 Civ. Proc., § 834(b).)

21 By contrast, nothing in SGMA vests GSAs, DWR, or the State Board with authority to
22 determine groundwater rights. In fact, SGMA expressly prohibits a groundwater sustainability
23 agency or plan from making “a binding determination of the water rights of any person or
24 entity.” (Wat. Code, § 10726.8(b); see also Wat. Code, § 10720.5(b) [“Nothing in [SGMA], or
25 in any groundwater management plan adopted pursuant to [SGMA], determines or alters surface
26 water rights or groundwater rights under common law or any provision of law that determines or
27 grants surface water rights.”]; Wat. Code, § 10720.1(b).) Nor does DWR approval of a
28 groundwater sustainability plan constitute a determination that a plan’s groundwater pumping

1 allocations are consistent with groundwater rights law. (Wat. Code, § 10738.)

2 California courts are also mandated to develop and impose a physical solution where one
3 is presented or to order a physical solution on its own motion. The trial court has both “the
4 authority and the duty to impose a physical solution on the parties in a comprehensive
5 adjudication where necessary and consistent with Article 2 [sic] of Section X [sic] of the
6 California Constitution.” (Code Civ. Proc., § 849(a) [emphasis added]; *Barstow, supra*, 23
7 Cal.4th at 1250 [citing *Lodi, supra*, 7 Cal.2d at 341]; *Rancho Santa Margarita, supra*, 11 Cal.2d
8 at 558-559; *Lodi, supra*, 7 Cal.2d at 341; *Peabody, supra*, 2 Cal.2d at 367-368, 383-384; *Santa*
9 *Maria I, supra*, 211 Cal.App.4th at 288; *Hillside, supra*, 205 Cal.App.4th at 538-539, 549.) This
10 authority and duty come directly from Article X, section 2 of the California Constitution.
11 (*Barstow, supra*, 23 Cal.4th at 1250 [citing *Lodi, supra*, 7 Cal.2d at 341]; *Rancho Santa*
12 *Margarita, supra*, 11 Cal.2d at 559.)

13 Moreover, trial courts have broad authority in developing physical solutions. “[The
14 California] Supreme Court has encouraged the trial courts to be creative in devising physical
15 solutions to complex water problems to ensure a fair result consistent with the constitution’s
16 reasonable-use mandate.” (*Santa Maria I, supra*, 211 Cal.App.4th at 288 [citing *Tulare Irr. Dist.*
17 *v. Lindsay-Strathmore Irr. Dist.* (1935) 3 Cal.2d 489, 574].) “It must be remembered that in this
18 type of case the trial court is sitting as a court of equity, and as such, possesses broad powers to
19 see that justice is done in the case.” (*Rancho Santa Margarita, supra*, 11 Cal.2d at 560.)

20 **6. THE COURT’S FINDINGS ON STORAGE ARE KEY TO DETERMINING SAFE**
21 **YIELD AND ULTIMATELY ADOPTING A PHYSICAL SOLUTION TO**
22 **ACHIEVE SUSTAINABLE GROUNDWATER MANAGEMENT.**

23 **A. Courts Determine Storage and Safe Yield and Retain Jurisdiction to**
24 **Redetermine Them as Conditions Change.**

25 Key to developing a fair, equitable, and workable physical solution are the amount of
26 Basin groundwater in storage and the Basin’s safe yield, with storage a necessary finding to
27 determining both safe yield and the physical solution. The California Supreme Court has defined
28 “safe yield” as “the maximum quantity of water which can be withdrawn annually from a ground

1 water supply under a given set of conditions without causing an undesirable result.” (*San*
2 *Fernando, supra*, 14 Cal.3d at 278; see also *Central & W. Basin Water Replenishment Dist. v.*
3 *So. Cal. Water Co.* (2003) 109 Cal.App.4th 891, 899 fn. 4.) Accordingly, a court determines
4 safe yield only after considering the physical characteristics of the basin, its size, the quantity of
5 groundwater in storage, and the potential that unregulated production would cause “undesirable
6 results.” (See *San Fernando, supra*, 14 Cal.3d at 278-279.)

7 In a groundwater rights adjudication, the court ultimately determines the amount of
8 groundwater in storage and establishes the safe yield, and then retains jurisdiction to revise them
9 to meet changed hydrologic conditions. (Code Civ. Proc., § 852; *San Fernando, supra*, 14
10 Cal.3d at 278, 287; *Pasadena, supra*, 33 Cal.2d at 937-938 [affirming court retains jurisdiction to
11 meet changing conditions, but reversing to eliminate a five-year limit on its power to review its
12 safe yield determination in favor of more frequent review “as the occasion may require”]; *Allen*
13 *v. Cal. Water & Tel. Co.* (1946) 29 Cal.2d 466, 482, 491-492 [reversing in part to amend
14 judgment to add court retains jurisdiction to revise safe yield as conditions change]; e.g.,
15 *Antelope Valley II, supra*, 62 Cal.App.4th at 1011-1012, 1038; *Hillside, supra*, 205 Cal.App.4th
16 at 541, 547.)

17 It is axiomatic that to determine storage and safe yield, trial courts must consider
18 technical data and expert opinion. Under long-established common law, courts have considered
19 technical data and expert opinion to determine storage and safe yield. (E.g., *San Fernando,*
20 *supra*, 14 Cal.3d at 278 [trial court considered computations of safe yield by State Board referee
21 and expert witnesses]; *Pasadena, supra*, 33 Cal.2d at 917 [finding issues to be tried were
22 dependent to a great extent on facts to be ascertained and reported by referee]; *Seaside, supra,*
23 183 Cal.App.4th at 474, *Antelope Valley II, supra*, 62 Cal.App.5th at 1011-1012.)

24 **B. Trial Courts Must Consider Technical Data and Expert Testimony to**
25 **Determine Storage and Safe Yield.**

26 The Streamlined Act and SGMA reinforce the jurisdiction of trial courts to consider
27 technical data and expert testimony as part of a comprehensive adjudication. (See *San*
28 *Fernando, supra*, 14 Cal.3d at 279 fn. 81; *Antelope Valley I, supra*, 59 Cal.App.5th at 251-252;

1 *Wright, supra*, 174 Cal.App.3d at 81.)

2 For example, the Streamlined Act expressly contemplates that the court, including
3 through the appointment of a special master, if selected, will “[i]nvestigat[e] technical and legal
4 issues” and “[c]onduct[] joint factfinding with the parties.” (Code Civ. Proc., § 845(a)(1) &
5 (a)(2).) Likewise, the Streamlined Act provides that the court may stay an adjudication to,
6 among other things, permit “development of technical studies that may be useful to the parties in
7 the comprehensive adjudication.” (Code Civ. Proc., § 848(a)(2).) The Streamlined Act also
8 provides, “Before adopting a physical solution, the court shall consider any existing groundwater
9 sustainability plan or program,” but a groundwater sustainability plan does not limit a court’s
10 affirmative authority and duty to impose a physical solution in the first instance. (Code Civ.
11 Proc., § 849(b).) And while the Streamlined Act “references SGMA and allows a court to
12 consider [SGMA], . . . it does not mandate that an adjudication action be consistent [with a
13 basin’s groundwater sustainability plan]. It says that a GSP may serve as the basis of a stipulated
14 judgment, but it doesn’t require it.” (Assem. Com. on Water, Parks, and Wildlife, Rep. on
15 Assem. Bill No. 1390 (20152016 Reg. Sess.) Apr. 14, 2015, p. 5 [emphasis added].)

16 Meanwhile, SGMA provides for DWR to consider, review, and comment upon any
17 physical solution contained in a judgment and empowers the trial court in an adjudication to
18 “determine whether to amend the judgment . . . to adopt [DWR’s] recommended corrective
19 actions,” or not. (Wat. Code, §§ 10737.4, 10737.6.) SGMA also permits any local agency to
20 submit to DWR “an alternative” to a groundwater sustainability plan. (Wat. Code, §§ 10721(n),
21 10733.6(a), 10737.4(b), 10737.4(c).) An alternative may include “[m]anagement pursuant to an
22 adjudication action.” (Wat. Code, § 10733.6(b)(2).) Indeed, previously adjudicated basins are
23 expressly exempt from requirements to form GSAs or to develop GSPs. (Wat. Code, § 10720.8.)

24 Moreover, nothing in SGMA changed the court’s authority to consider technical data and
25 expert testimony. The statutorily granted jurisdiction that groundwater sustainability agencies,
26 DWR, and the State Board have over certain water management activities does not deprive the
27 superior courts of jurisdiction to consider technical data in a comprehensive adjudication as they
28 have for decades. (See *Int’l Assn. of Fire Fighters v. Pub. Employment Relations Bd.* (2011) 51

1 Cal.4th 259, 270 [Courts “will not infer a legislative intent to entirely deprive the superior courts
2 of judicial authority in a particular area; the Legislature must have expressly so provided or
3 otherwise clearly indicated such an intent.”]; see, e.g., *Environmental Defense Fund v. East Bay*
4 *Mun. Util. Dist.* (1980) 26 Cal.3d 183, 200 [confirming the concurrent jurisdiction of the State
5 Board and the courts to implement Article X, section 2’s reasonable use mandate]; *San*
6 *Fernando, supra*, 14 Cal.3d at 278; *Antelope Valley II, supra*, 62 Cal.App.5th at 1011-1012.)
7 And, both the Streamlined Act and SGMA confirm that neither alters the long-developed
8 common law on water rights. (Code Civ. Proc., § 830(b)(7) [“Except as provided in this
9 paragraph, this chapter shall not alter groundwater rights or the law concerning groundwater
10 rights.”]; Wat. Code, § 10720.5 [“Nothing in this part, or in any groundwater management plan
11 adopted pursuant to this part, determines or alters surface water rights or groundwater rights
12 under common law or any provision of law that determines or grants surface water rights.”].)

13 Case law further supports the continuing jurisdiction of trial courts to consider technical
14 data and expert witness testimony in any comprehensive adjudication. In *California American*
15 *Water v. City of Seaside* (2010) 183 Cal.App.4th 471, the water management district contended
16 the trial court exceeded its jurisdiction and violated the separation of powers doctrine by
17 adopting and ultimately enforcing a physical solution in an adjudication. (*Id.* at 473-474.) The
18 district contended that the court’s imposition of a physical solution interfered with the district’s
19 exclusive authority to regulate groundwater pumping and adopt a groundwater management plan
20 for the basin. (*Id.* at 475-476.) The appellate court disagreed, holding that the “[trial] court
21 acted within its jurisdiction and properly exercised its discretion in adhering to its prior rulings to
22 minimize conflict with and frustration of the physical solution.” (*Id.* at 481.) The appellate court
23 quoted the trial court with approval, “Clearly, the [L]egislature contemplated that courts had the
24 power to develop management plans for aquifer management even if a water management
25 district already existed in a geographical area.” (*Id.* at 476, 481-482 [emphasis added].)

1 C. **The Jurisdiction of Trial Courts to Consider Technical Data Through**
2 **Adjudication Concurrent with Groundwater Sustainability Agencies, DWR,**
3 **and the State Board Promotes Sustainable Groundwater Management.**

4 SGMA and the Streamlined Act work together to achieve sustainable groundwater
5 management consistent with the constitutional mandate of maximizing reasonable and beneficial
6 use of water. SGMA requires courts to manage comprehensive adjudications under the
7 Streamlined Act in a manner “consistent with the attainment of sustainable groundwater
8 management.” (Wat. Code, § 10737.2.) SGMA defines “sustainable groundwater management”
9 as “the management and use of groundwater in a manner that can be maintained during the
10 planning and implementation horizon without causing undesirable results.” (Wat. Code,
11 § 10721(v).) This definition does not mention sustainable yield and does not require a court
12 managing an adjudication to blindly and without question adopt as its determination of safe yield
13 whatever figure the groundwater sustainability plan has determined as the basin’s sustainable
14 yield. Consistent with SGMA, the Streamlined Act, and parties’ due process rights to discovery
15 and cross-examination of witnesses in establishing water rights and a physical solution, a trial
16 court must consider all technical data, including a groundwater sustainability plan’s conclusion
17 of “sustainable yield,” to reach its own determination of safe yield. (See Assem. Com. on Water,
18 Parks, and Wildlife, Rep. on Assem. Bill No. 1390 (2015-2016 Reg. Sess.) Apr. 14, 2015, p. 5 [a
19 GSP may serve as the basis for a stipulated judgment, but the law “doesn’t require it”].)

20 In adopting SGMA, the California Legislature knew and understood that common law
21 empowers courts to set the safe yield, not just to manage a safe yield number. (E.g., *San*
22 *Fernando*, *supra*, 14 Cal.3d at 287; *Hillside*, *supra*, 205 Cal.App.4th at 541; *Seaside*, *supra*, 183
23 Cal.App.4th at 476, 481.) Notably, the California Legislature uses “sustainable yield” in SGMA.
24 (E.g., Wat. Code, § 10721(w) [defining “sustainable yield”].) Meanwhile, the long-established
25 common law of groundwater adjudications uses “safe yield.” (E.g., *San Fernando*, *supra*, 14
26 Cal.3d at 278 [defining “safe yield”]; *Pasadena*, *supra*, 33 Cal.2d at 922; *Antelope Valley II*,
27 *supra*, 62 Cal.App.5th at 1001.) Moreover, with one express exception regarding evidence of
28 prescription under SGMA, the California Legislature explicitly left the common law in place

1 under SGMA and the Streamlined Act. (Code Civ. Proc., § 830(b)(7) [“Except as provided in
2 this paragraph, this chapter shall not alter groundwater rights or the law concerning groundwater
3 rights.”]; Wat. Code, § 10720.5(a) [“Nothing in this part modifies rights or priorities to use or
4 store groundwater consistent with Section 2 of Article X of the California Constitution . . .”].)

5 A trial court’s concurrent consideration of technical data also does not create redundancy,
6 impose unnecessary costs, or render the groundwater sustainability agency’s technical data
7 inconsequential. (See Wat. Code, § 10737.2.) Rather, the court’s concurrent consideration of
8 technical data through the comprehensive adjudication process promotes reliance on the best
9 available science and vetting the best possible management solution. (See, e.g., *Pasadena*,
10 *supra*, 33 Cal.2d at 919 [approving appointment of former Division of Water Resources as
11 referee to prepare report on technical issues and parties’ opportunity to introduce evidence
12 contrary to the facts appearing in the referee’s report and to oppose the report]; see also *People*
13 *ex rel. Brown v. Tri-Union Seafoods, LLC* (2009) 171 Cal.App.4th 1549, 1575 [“findings based
14 on scientific inquiry and research can easily become dated and outmoded as science develops
15 and new research explains the phenomena in question more thoroughly and completely.
16 ‘Science . . . represents a process for proposing and refining theoretical explanations about the
17 world that are subject to further testing and refinement. . . . Scientific conclusions are subject to
18 perpetual revision.’” [quoting *Daubert v. Merrell Dow Pharmaceuticals, Inc.* (1993) 509 U.S.
19 579, 590]].) Simply stated: the Court must consider technical data from qualified experts to
20 evaluate Basin conditions necessary to establish a judgment and physical solution for the Basin.

21 **7. CONCLUSION**

22 District respectfully requests that the Court (a) immediately issue an order to show cause
23 to confirm the Basin boundary consistent with Bulletin 118; (b) divide trial into phases; (c) set
24 the Phase 1 Trial and define the scope of issues to consist of the amount of groundwater in
25 storage, including the amount of available fresh water in storage; and the federal reserved water
26 right claim; (d) lift the stay on discovery, but only as to the issues to be tried during the Phase 1
27 Trial; and (e) lift the stay on expert disclosures and set a deadline for the exchange of expert
28 disclosures, but only as to the issues to be tried during the Phase 1 Trial.

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DATED: February 23, 2024

MURPHY & EVERTZ LLP



By: _____
Douglas J. Evertz
Emily L. Madueno
Attorneys for Defendant, Cross-Complainant, &
Cross-Defendant
INDIAN WELLS VALLEY WATER DISTRICT

DECLARATION OF DOUGLAS J. EVERTZ

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DECLARATION OF DOUGLAS J. EVERTZ

I, Douglas J. Evertz, declare:

1. I am an attorney, a member of the State Bar of California, and authorized to practice law in California. I am a partner of Murphy & Evertz LLP, 650 Town Center Drive, Suite 550, Costa Mesa, California 92626. My firm serves as co-counsel of record for Defendant, Cross-Complainant, and Cross-Defendant Indian Wells Valley Water District (“District”) in the above-captioned case and related cases. I have personal knowledge of the facts set forth below.

2. On June 16, 2021, District initiated a comprehensive adjudication of the Indian Wells Valley Groundwater Basin (“Basin”) when, on District’s behalf, my office filed a Cross-Complaint for Comprehensive Adjudication pursuant to the California Streamlined Groundwater Adjudication Act (Code Civ. Proc., §§ 830-852).

3. I have represented clients in prior groundwater adjudications, including the Mojave River Basin Adjudication and the Antelope Valley Basin Groundwater Adjudication. In both of those adjudications, a group of the major pumpers formed a group of technical consultants to work together using the best available science in furtherance of a negotiated settlement. In those adjudications, the respective trial courts found the negotiated settlements workable and ultimately adopted them as the physical solutions for the basins. I sought to follow this tried-and-true practice here on behalf of District.

4. In 2022, I directed District’s technical consultants—groundwater professionals from Krieger & Stewart Engineering Consultants, Parker Groundwater, and Ramboll—to work with technical consultants for the other major pumpers in the Basin. I communicated with counsel for the United States and invited the United States to participate in this collaborative technical group.

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I declare under penalty of perjury under the laws of the State of California that the foregoing is true and correct.

Executed this 21st day of February 2024, at Costa Mesa, California.



Douglas J. Evertz

**DECLARATION OF
TIMOTHY K. PARKER, PG, CEG, CHG**

1 **DECLARATION OF TIMOTHY K. PARKER, PG, CEG, CHG**

2 I, Timothy K. Parker, PG, CEG, CHG, declare:

3 1. I am a licensed professional geologist (license no. 5584), certified engineering
4 geologist (license no. 1926), and certified hydrogeologist (license no. 0012) in California. I have
5 worked as an independent technical consultant with Parker Groundwater since 2009. I have also
6 worked with Ramboll Americas Engineering Solutions, Inc., since 2021, as the consultant
7 Project Director for the California Department of Water Resources (“DWR”) statewide airborne
8 geophysics aquifer mapping project. I have knowledge of the facts set forth below, and, if called
9 as a witness, I could and would testify to them.

10 2. I have more than 35 years of hydrogeologic experience in the field of
11 groundwater supply and groundwater resources management. Before working with Parker
12 Groundwater and Ramboll, I previously worked for Schlumberger Water Services, applying best
13 available science oilfield service technologies to the groundwater industry, and for DWR.

14 3. I also have dedicated, and continue to dedicate, significant volunteer time to the
15 groundwater industry through outside professional organizations. I currently serve or have
16 served as a director on the National Ground Water Association, American Groundwater Trust,
17 International Association of Hydrogeologists, California Groundwater Coalition, and
18 Groundwater Resources Association of California (“GRA”). I was the GRA Legislative
19 Committee Chairman for twenty years and, during that time, I became involved with legislation
20 and policy development in Sacramento. Through GRA, I worked with state legislators to
21 develop science-based policy and laws, and I was involved in the development of the Sustainable
22 Groundwater Management Act (California Water Code section 10720 *et seq.*, “SGMA”).

23 4. I have developed extensive experience in all aspects of groundwater supply and
24 resource management, including SGMA, groundwater sustainability plans (GSPs), groundwater
25 management plans, groundwater and surface water computer modeling, geophysical applications
26 for groundwater characterization, groundwater and surface water rights, groundwater recharge,
27 well construction and management, water quality regulatory requirements and compliance,
28 groundwater contaminant characterization and mitigation, national discharge pollution

1 eliminations system (NPDES) and surface water discharges, recycled water reuse regulations and
2 compliance, conjunctive-use of surface water and groundwater, California Environmental
3 Quality Act (CEQA) requirements and compliance with cumulative impacts, and facilitation of
4 diverse stakeholders.

5 5. Since 2010, I have served as a consulting hydrogeologist for the Indian Wells
6 Valley Water District (“District”). The District is the main public water supplier in the Indian
7 Wells Valley Groundwater Basin (“Basin”), providing water to the community of Ridgecrest and
8 surrounding areas. As the District’s consulting hydrogeologist, I support the District in various
9 groundwater-related activities, including providing hydrogeologic technical support on the
10 comprehensive adjudication, and prior to that, serving as the District’s representative on the
11 Technical Advisory Committee (“TAC”) to the Indian Wells Valley Groundwater Authority
12 (“IWVGA”) and advising the District on SGMA, the water supply improvement plan (WSIP),
13 and other groundwater technical and policy matters. I withdrew from the TAC on May 18, 2022.

14 6. As the Basin’s groundwater sustainability agency under SGMA, IWVGA
15 prepared a groundwater sustainability plan (“Basin GSP”) and submitted it to DWR in 2020. I
16 have reviewed and I am familiar with the Basin GSP and DWR’s response to it in a January 13,
17 2022 letter to IWVGA. DWR conditionally approved the Basin GSP in 2022. DWR noted
18 satisfaction with SGMA objectives and “substantial compliance” with SGMA regulations, and
19 included seven recommended corrective actions. DWR’s response to the Basin GSP also
20 recognized ongoing litigation challenging the Basin GSP and stated, “This assessment is limited
21 to technical review of the submitted Plan, as required by SGMA and is not intended and should
22 not be read as a comment on the litigation or the legal or factual claims raised by the parties.”
23 DWR has acknowledged in public forums that it has insufficient capacity for an in-depth
24 technical review of the GSPs submitted to it and, hence, uses the term “substantial compliance.”
25 DWR has indicated that while GSPs may be conditionally approved, DWR expects plans will
26 improve over time with additional data collection and data gap filling. DWR also indicated that
27 plans conditionally approved today could be deemed incomplete or inadequate during future
28 reviews. For example, on November 29, 2022, I attended a committee meeting of the

1 Groundwater Committee of the Association of California Water Agencies (“ACWA”) as part of
2 ACWA’s 2022 Fall Conference in Indian Wells, California. I recall Mallory Boyd, a Director of
3 the District, who also attended the meeting, asking the DWR representative about the meaning of
4 DWR’s review of GSPs, and whether DWR reviewed the Basin’s GSP in particular for technical
5 accuracy. I recall the response from the DWR representative indicating that DWR’s approval of
6 a GSP does not bless everything in the plan and that DWR’s focus is on “substantial compliance”
7 with its regulatory requirements for GSPs.

8 7. In 2022, I began meeting regularly with a group of experienced and qualified
9 groundwater professionals (“Technical Working Group”) designated by individuals and entities
10 accounting for more than 80% of the Basin’s total groundwater production in Water Year 2022
11 as estimated by the Technical Working Group. I met with the Technical Working Group
12 members roughly every other week for over a year, and we continue to meet.

13 8. The Technical Working Group is composed of the following groundwater
14 professional firms:

- 15 (a) Parker Groundwater;
- 16 (b) Ramboll;
- 17 (c) Krieger & Stewart Engineering Consultants (“Krieger & Stewart”);
- 18 (d) Aquilogic, Inc. (“Aquilogic”);
- 19 (e) Geoscience Support Services, Inc. (“Geoscience”); and
- 20 (f) Luhdorff & Scalmanini Consulting Engineers (“LSCE”).

21 9. Each member of the Technical Working Group was appointed by an active party
22 in the Indian Wells Valley Groundwater Basin Adjudication, Case No. 30-2021-01187275-CU-
23 OR-CJC. In this case, the District seeks a comprehensive groundwater basin adjudication under
24 the California Streamlined Groundwater Adjudication Act (“Streamlined Act”).

25 10. The District appointed Parker Groundwater, Ramboll, and Krieger & Stewart to
26 the Technical Working Group. I serve on the Technical Working Group from Parker
27 Groundwater and Ramboll. Alka Singhal also serves on the Technical Working Group from
28

1 Ramboll. Charles A. Krieger, P.E., serves on the Technical Working Group from Krieger &
2 Stewart.

3 (a) I founded Parker Groundwater in 2009. We provide public and private
4 clients with groundwater resources management, including water resources planning, policy
5 consulting, groundwater management planning and program implementation, groundwater
6 resources development, and facilitation services.

7 (b) Ramboll is a global, multi-disciplinary consultancy firm. It was founded
8 in Denmark in 1945 and operates across 35 countries. Ramboll's groundwater expertise includes
9 groundwater resources management, groundwater mapping and analysis, geophysical surveys,
10 3D geological modeling, groundwater modeling, and well field exploration strategies.

11 (c) Ms. Singhal is a senior managing consultant at Ramboll. She has more
12 than 15 years of consulting experience in groundwater modeling, implementing hydrogeologic
13 studies, and spatial analysis tools for Ramboll's Site Solutions projects. She has expertise in
14 assessing the reliability and security of water resources, water rights, and the resiliency of water
15 supply for manufacturing and industrial sites, food and beverage makers, recreational sites, and
16 energy providers.

17 (d) Krieger & Stewart was founded in 1971. Since then, the firm has
18 provided civil and environmental engineering consulting services to public agency clients
19 throughout California, including the District. Krieger & Stewart has served as the District's
20 consulting engineer since 1976.

21 (e) Mr. Krieger is a registered professional civil engineer (RCE 44545) in
22 California and has been so licensed since 1989. He has served as president and chief executive
23 officer of Krieger & Stewart since 2011 and has been employed at Krieger & Stewart since 1986.
24 Since 1992, he has served as the principal consulting engineer to the District from Krieger &
25 Stewart. His primary field of practice is consulting engineering for special districts and
26 municipalities, primarily with regard to water supplies and water systems (production, treatment,
27 transmission, storage, and distribution). He has extensive experience in all facets of groundwater
28 resource management, including evaluating/identifying well locations, designing production and

1 monitoring wells (diameter, depth, screened interval, casing material), prescribing well
2 maintenance programs, and analyzing groundwater response to pumping activities. He also has
3 significant experience in well design, construction, testing, operation, maintenance, and
4 rehabilitation throughout Southern California and primarily in desert regions, including in Inyo,
5 Kern, San Bernardino, Riverside, Los Angeles, Imperial, and San Diego counties.

6 11. Mojave Pistachios, LLC, John Thomas Conaway, John Thomas Conaway Trust,
7 John Thomas Conaway Living Trust u/d/t August 7, 2008, Nugent Family Trust, and Sierra
8 Shadows Ranch LP (collectively, "Mojave Pistachios") appointed Aquilogic to the Technical
9 Working Group. Anthony Brown and Wade Major, MBA, P.E., serve on the Technical Working
10 Group from Aquilogic.

11 (a) Aquilogic was established in 2011 by founder, CEO, and Principal
12 Hydrologist, Anthony Brown. The Aquilogic team has been working in the Basin for
13 approximately a decade.

14 (b) Mr. Brown is the founder of and CEO and principal hydrologist at
15 Aquilogic. He has over 30 years of experience in many aspects of infrastructure engineering and
16 environmental consulting, with a focus on hydrologic science, water resources, environmental
17 engineering, and water treatment and supply. He has managed or directed an extensive number
18 of water resources projects, including: (i) assessment and development of groundwater resources;
19 (ii) development of water budgets; (iii) determination of safe yield and sustainable yield;
20 (iv) water rights disputes, including basin adjudications; (v) preparation and review of GSPs in
21 accordance with SGMA; (vi) evaluation of undesirable results from groundwater production;
22 (vii) assessment of groundwater-surface water interaction that supports groundwater-dependent
23 ecosystems; (viii) evaluation of water quality concerns, notably from agricultural and industrial
24 pollutants; (ix) development of numerical groundwater flow models; and (x) preparation of
25 physical solutions in groundwater basin adjudications. Mr. Brown has also provided expert
26 testimony in numerous cases in both Federal and State court and has been retained as an expert
27 witness in water rights disputes in basins including the Napa, Paso Robles, Santa Maria,
28 Cuyama, Goleta, Ojai, Las Posas, Oxnard Plain, Pleasant Valley, Borrego, and Mojave

1 groundwater basins. He has also testified before the U.S. Congress on groundwater issues and
2 briefed White House staff, federal, state, and local elected officials and regulators, independent
3 commissions, professional groups, academic institutions, and the news media (including CBS 60
4 Minutes, National Public Radio [NPR], and local newspapers) on groundwater and pollution
5 issues. Mr. Brown holds a Masters degree in Engineering Hydrology, postgraduate diploma in
6 Civil Engineering from Imperial College London, and a Bachelors degree in Geography from
7 King's College London.

8 (c) Mr. Major is a senior consultant at Aquilogic. He received his Bachelors
9 and Masters degrees in Civil Engineering from the University of Alberta, and an MBA in
10 Executive Management from Royal Roads University. Mr. Major is a Professional Civil
11 Engineer in the State of California and the Province of British Columbia. Together with
12 Mr. Brown, Mr. Major and others on the Aquilogic team are currently providing consulting
13 services related to SGMA and other groundwater management matters in approximately 40
14 basins in California. These include numerous basins in the San Joaquin Valley and desert
15 southwest. This experience is in addition to Aquilogic's work on matters in more than 20
16 additional basins in California. Aquilogic prepared the GSP for the western portion of the Kern
17 Sub-basin and holds a seat on several technical advisory committees in several other basins
18 subject to SGMA.

19 12. Searles Valley Minerals Inc. appointed Geoscience to the Technical Working
20 Group. Dr. Johnson Yeh and Lauren Wicks, PG, serve on the Technical Working Group from
21 Geoscience.

22 (a) Geoscience was founded in the 1960s and, today, comprises an esteemed
23 team of engineers, geologists, and hydrologists focusing on groundwater resource needs.

24 (b) Dr. Yeh is a principal and lead groundwater modeler at Geoscience.
25 During the nearly three decades of his career, he has been involved in the project management of
26 Geoscience's groundwater modeling efforts, including some of its most high-profile
27 geohydrologic investigations, groundwater basin/water quality studies, and artificial recharge
28 projects. Dr. Yeh also possesses a broad knowledge of GIS tools utilized in support of modeling

1 work and oversees Geoscience’s GIS staff members. Dr. Yeh spearheads the generation and
2 quality control of technical documents and technical presentations to both clients and regulators,
3 including leading training and workshops on the use of models to the USGS and United States
4 Environmental Protection Agency.

5 (c) Ms. Wicks is a professional geohydrologist at Geoscience. She has more
6 than a decade of experience with groundwater and environmental investigations performed for
7 numerous municipalities, state agencies, and private clients throughout California. Her expertise
8 includes groundwater flow and transport modeling, geohydrologic investigations, groundwater
9 basin and water management studies, and technical report preparation. She has also been
10 involved with artificial recharge projects, litigation projects, and GIS applications.

11 13. Meadowbrook Dairy Real Estate, LLC, Big Horn Fields, LLC, Brown Road
12 Fields, LLC, Highway 395 Fields, LLC, and the Meadowbrook Mutual Water Company
13 (collectively, “Meadowbrook”) appointed LSCE to the Technical Working Group. Eddy
14 Teasdale, PG, CHG, and Will Halligan, PG, serve on the Technical Working Group from LSCE.

15 (a) LSCE was founded in 1980. LSCE’s team of engineers, geologists,
16 hydrogeologists, and hydrologists have extensive experience in groundwater management
17 matters.

18 (b) Mr. Teasdale is a principal hydrogeologist at LSCE. He has over 20 years
19 of experience working on geological and hydrogeological investigations in the United States and
20 internationally (England, Ireland, North Africa, and Guam) conducting groundwater basin
21 analyses, including assessing hydrologic budgets and performing groundwater contamination and
22 remediation analyses. He is a professional geologist and certified hydrogeologist in California.
23 He holds a Bachelor of Science in Geology from the University of Texas and a Master of
24 Science in Hydrogeology from the University of Idaho. His projects have involved complex,
25 comprehensive geology, hydrogeology, conveyance, flood control, and environmental issues.
26 He has worked in all major aquifer types (alluvial basins, volcanic, carbonate, and bedrock
27 terrains). Mr. Teasdale’s technical expertise includes hydrogeologic characterization and
28 groundwater modeling. He has served as a subject matter expert for the Professional Geologist

1 and Certified Hydrogeologist exams for the Department of Consumer Affairs in California since
2 2006. He has assisted in the development, review, grading, and appeals process for the annual
3 hydrogeologist certification exam. Participation in the six-member expert team is by invitation
4 only and participants are required to be both licensed and certified in their specialties in addition
5 to having demonstrated extensive applied experience in their respective fields.

6 (c) Mr. Halligan is a registered Professional Geologist in California with over
7 30 years of professional experience. He is the Senior Principal Hydrogeologist and President of
8 LSCE. Mr. Halligan has extensive experience in groundwater management and development,
9 groundwater resource investigations and monitoring, groundwater modeling, well design, and
10 environmental analysis, particularly in the arid basins of California. His experience includes
11 evaluation and feasibility of groundwater recharge projects such as surface recharge ponds,
12 aquifer storage and recovery (ASR) projects, and the influence of these projects on groundwater
13 storage and quality. Mr. Halligan has also conducted investigations evaluating the impact of
14 multiple management actions and projects on regional and groundwater conditions. Mr. Halligan
15 was also involved in SGMA GSP regulation development as part of Groundwater Resources
16 Association's Sustainable Groundwater Management Committee. From that work, he advises
17 Groundwater Sustainability Agencies (GSAs) and stakeholder pumpers on GSP development and
18 implementation. He has also taught groundwater resources related classes for University of
19 California Extension program with a focus on groundwater management and development,
20 SGMA, and GSP development and implementation.

21 14. As a member of the Technical Working Group, it is my understanding that our
22 initial assignment was to estimate the total amount of groundwater and fresh groundwater in
23 storage within the Basin and the safe yield of the Basin. To complete our assignment, we
24 analyzed all available data relating to the Basin and Basin groundwater, including the Basin
25 GSP, we performed additional analyses, and we sought to rely on the best available science. The
26 Basin boundary is defined by DWR in its Bulletin 118 report as the Indian Wells Valley
27 Groundwater Basin, DWR Basin No. 6-54.

1 15. As to the amount of groundwater in storage, I participated with the other members
2 of the Technical Working Group in the preparation of a report entitled, “Technical Working
3 Group: Initial Assessment of Groundwater Storage for the Indian Wells Valley Groundwater
4 Basin” (“Storage Paper”). I have reviewed and I am familiar with the Storage Paper. Attached
5 as Exhibit “A” is a true and correct copy of the Storage Paper.

6 16. To estimate the total amount of groundwater in storage, we defined the physical
7 parameters of the Basin as defined by DWR, including its geologic and hydrologic
8 characteristics. We then considered and applied three separate scientific methodologies: (a) the
9 Desert Research Institute (“DRI”) groundwater flow model domain and framework based on
10 available information paid for by the United States Navy and used by IWVGA to develop the
11 Basin’s GSP; (b) the Ramboll Hydrogeologic Conceptual Framework for the China Lake Area of
12 the Basin; and (c) the Ramboll Hydrogeologic Conceptual Framework using best available
13 science, airborne electromagnetics (AEM), as is being applied statewide by DWR, and as
14 modified by the incorporation of Basin-wide seismic reflection data.

15 17. The Technical Working Group’s conclusion of the average groundwater volumes
16 estimated from the three scientific methods we considered are: (a) the total volume of
17 groundwater in storage is approximately 66.9 million AF; and (b) the amount of fresh
18 groundwater in storage is approximately 37.5 million AF, at a minimum.

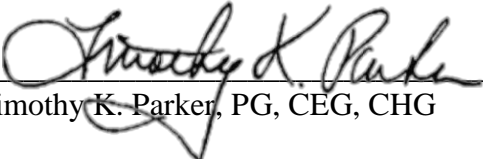
19 18. IWVGA used a 1993 Bureau of Reclamation report to estimate storage. In other
20 words, IWVGA presented no contemporary work on storage. The Bureau estimated that there
21 were anywhere from 1,020,000 AF to 3,020,000 AF of groundwater in storage underlying only a
22 92.5-square-mile (59,200 acres) portion of the Basin. The Bureau’s storage estimate is based on
23 only 15 percent of the Basin’s total surface area and the assumption of useable water in the 100
24 to 300 feet of saturated aquifer below groundwater contour levels. IWVGA did not use the DRI
25 groundwater flow model to estimate total storage. Instead, IWVGA used the DRI groundwater
26 flow model only to estimate change in storage of 620,000 AF from 1992 to 2017. IWVGA then
27 estimated the remaining readily available fresh groundwater in storage to be 2,370,000 AF less
28 620,000 AF, or 1,750,000 AF. The difference of approximately 35.75 million AF between the

1 Technical Working Group's estimate of fresh groundwater in storage (a minimum of
2 37.5 million AF) and IWVGA's estimate (1.75 million AF) reflects IWVGA's use of incomplete,
3 outdated data while the Technical Working Group used the best available science and data.

4 19. It is my understanding that the Technical Working Group's current assignment is
5 to assist in developing potential basin management strategies and a proposed physical solution
6 that would maximize beneficial use of Basin groundwater while respecting water rights without
7 causing undesirable results, consistent with the Streamlined Act and SGMA.

8 I declare under penalty of perjury under the laws of the State of California that the
9 foregoing is true and correct.

10 Executed this 23rd day of February 2024, at Sacramento, California.

11 
12 _____
13 Timothy K. Parker, PG, CEG, CHG

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EXHIBIT A

**ASSESSMENT OF
GROUNDWATER STORAGE
FOR THE INDIAN WELLS VALLEY
GROUNDWATER BASIN**

**Prepared by:
Indian Wells Valley Technical Working Group**

February 23, 2024

**TECHNICAL WORKING GROUP:
ASSESSMENT OF GROUNDWATER STORAGE FOR THE INDIAN WELLS VALLEY
GROUNDWATER BASIN**

Table of Contents

1.0	Executive Summary	1
2.0	Introduction	2
2.1	Background	2
2.2	Terms and Definitions.....	2
2.2.1	Definition of Storage	2
2.3	Basin Setting	4
2.3.1	Hydrostratigraphy	5
2.3.2	Basin Hydraulic Properties.....	7
3.0	Groundwater Storage Volume – Estimate 1	9
3.1	Methodology	9
3.2	Results.....	11
3.3	Additional Considerations	12
4.0	Groundwater Storage Volume – Estimate 2	13
4.1	Methodology	13
4.2	Results.....	14
4.3	Additional Considerations	15
5.0	Groundwater Storage Volume – Estimate 3	16
5.1	Methodology	16
5.1.1	Updating the Hydrogeological Conceptual Framework (HCF).....	16
5.1.2	Determining the Percentage of Net Sand and Net Clay	18
5.1.3	Specific Yield and Storativity	21
5.2	Results.....	22
5.3	Additional Considerations	25
6.0	Discussion	26
6.1	Total Groundwater in Storage	26

6.2 Fresh and Brackish / Saline Groundwater in Storage 27

6.3 Groundwater Volume Differences Between The Various Methodologies 28

6.4 Estimated Groundwater Volumes Compared To The GSP 29

7.0 Conclusions 30

8.0 References 32

Figures

No.	Description
<i>(Attached)</i>	
1	Area Map
2	Indian Wells Valley Basin Depositional Environment
3	2-Aquifer Conceptualization
4	2-Aquifer West-East Cross-Section
5	USBR Logs Showing Lower Permeability Sediments
6	3-Aquifer Conceptualization
7	Recent Basin Cross-Sections
8	Basin Cross-Section B-B'
9	Basin Cross-Section E-E'
10	Brown & Caldwell CHM Geometry
11	DRI Model Layers
12	Ramboll 4-Zone HCM
13	DRI Model Boundary
14	Spring 2015 Groundwater Surface Elevation
15	Cross-Section Locations
16	DRI Model Cross-Section A-A'
17	DRI Model Cross-Section B-B'
18	DRI Model Cross-Section C-C'
19	DRI Model Cross-Section D-D' & E-E'
20	Overlap of Surfer Model Layers
21	DRI Model Specific Yield (Sy) Distribution
22	DRI Model Distribution of Total Dissolved Solids (TDS)
23	Layer 1 Estimated Volumes
24	Layer 2 Estimated Volumes
25	Layer 3 Estimated Volumes
26	Layer 4 Estimated Volumes
27	Layer 5 Estimated Volumes
28	Layer 6 Estimated Volumes

29 Groundwater Storage Calculation Areas
 30 HGZ 1 Extent and Thickness
 31 HGZ 2 Extent and Thickness
 32 HGZ 3 Extent and Thickness
 33 Location of the Reprocessed Seismic Lines in the Indian Wells Valley Basin
 34 Elevation of the Basement in the Indian Wells Valley Basin
 35 Cross-Sections Showing the Updated HCF, Including the Four HGZs, & Location Map
 36 Seismic Section 92-02, as Interpreted in the Updated HCF
 37 Total Thickness of the Sediments in the Indian Wells Valley Basin
 38 Thickness of Each HGZ in the Updated HCF
 39 Example Showing How the Net Sand (Coarse Materials) was Interpreted
 40 Net Sand for HGZ1, Represented in One-Mile Square Grids
 41 Net Sand for HGZ2, Represented in One-Mile Square Grids
 42 Seismic Line 92-02 Showing the Interpreted Net Sand (Dark Red Colors)
 43 Seismic Line 92-02 Showing the Interpreted Net Clay (Dark Brown Colors)
 44 Distribution of Minimum Total Groundwater in Storage in HGZ1
 45 Distribution of Minimum Total Groundwater in Storage in HGZ2
 46 Distribution of Minimum Total Groundwater in Storage in HGZ3
 47 Distribution of Minimum Total Groundwater in Storage in HGZ4
 48 Distribution of Total Groundwater in Storage Under 1,000 mg/L TDS
 49 Comparison of Estimate 1 Lithology to Estimate 3 Lithology

Tables

No.	Description	Page
1	Estimates of Volumes of Fresh, Brackish / Saline, and Total Water in Storage within the DRI Model Domain	12
2	Estimates of Volume of Water within HGZ1.....	14
3	Estimates of Volume of Water within HGZ2.....	14
4	Estimates of Volume of Water within HGZ3.....	15
5	Estimates of Volume of Water within Each HGZ and the Basin	15
6	Total Area and Thickness of the HGZs	18
7	Calculation of the Percent Net Sand, Mixed Lithology and Net Clay for HGZ1 and HGZ2	19
8	Calculated Net Sand in HGZ3 and HGZ4, Averaged for Each Seismic Line (Collier Geophysics, 2021 and 2023)	20
9	Calculation of the Total Volume, Net Sand Volume and Mixed/Fines (Lithology with Mixed with or Only Containing Silt and/or Clay) for Each HGZ	21

10	Range of Specific Yield from USGS (1967)	21
11	Calculated Total Groundwater in Storage for Each HGZ.....	23
12	Minimum and Maximum Total Groundwater in Storage Values Divided up by Basin Area.....	23
13	Minimum and Maximum Storage Value for Each HGZ, Divided Up Into Water Quality Zone, with Respect to Total Dissolved Solids (TDS) Concentrations	24
14	Key Differences Between the Three Groundwater Storage Estimates.....	26
15	Total Groundwater Volumes in Storage by Estimate and HGZ.....	27
16	Total Fresh and Brackish / Saline Groundwater Volumes in Storage by Estimate and HGZ.....	28
17	Total Groundwater Storage Volumes Using Estimate 1 Methodology with Different Sy	29

Acronyms, Abbreviations, and Initialisms

Abbreviation	Description
3D	3 dimensional
%	percent or percentage
<	less than
>	greater than
=	equal
AEM	airborne electro-magnetic
AF	acre-feet
amsl	above mean sea level
b	aquifer thickness
Basin	Indian Wells Valley Groundwater Basin
bgs	below ground surface
CASGEM	California Statewide Groundwater Elevation Monitoring
CHM	Conceptual Hydrogeologic Model
DHZ	Deep Hydrogeologic Zone
District	Indian Wells Valley Water District
DRI	Desert Research Institute
DTW	depth to water
DWR	California Department of Water Resources
Eqn	equation
ESRI	Environmental Systems Research Institute
ft	feet
ft ⁻¹	per foot
ft/d	feet per day
GDE	groundwater dependent ecosystem(s)
Geoscience	Geoscience Support Services, Inc.
GIS	Geographic Information System(s)
GRD	Grid files
GSP	Groundwater Sustainability Plan
HCF	Hydrogeologic Conceptual Framework
HCM	Hydrogeologic Conceptual Model
HGZ	Hydrogeologic Zone
HGZ1	Hydrogeologic Zone 1
HGZ2	Hydrogeologic Zone 2
HGZ3	Hydrogeologic Zone 3
HGZ4	Hydrogeologic Zone 4

IHZ	Intermediate Hydrogeologic Zone
IWV	Indian Wells Valley
IWVGA	Indian Wells Valley Groundwater Authority
K	hydraulic conductivity
Kx	horizontal hydraulic conductivity
Kz	vertical hydraulic conductivity
KCWA	Kern County Water Agency
K&S	Krieger & Stewart Engineering Consultants
LSCE	Luhdorff & Scalmanini Consulting Engineers
mg/L	milligrams per liter
ohm-m	ohm-meter – unit of electrical resistivity
PEST	Parameter Estimation Software
S	storativity
SHZ	Shallow Hydrogeologic Zone
SNORT	Supersonic Naval Ordnance Research Track
Ss	specific storage
Surfer	Surfer 21.2.192 (Surfer™)
Sy	specific yield
TDS	total dissolved solids
TWG	Technical Working Group
U.S.	United States
USBR	United States Bureau of Reclamation
USGS	United States Geological Survey
USN	United States Navy
XYZ	coordinates in the X,Y,Z reference frame

TECHNICAL WORKING GROUP: ASSESSMENT OF GROUNDWATER STORAGE FOR THE INDIAN WELLS VALLEY GROUNDWATER BASIN

1.0 Executive Summary

A Technical Working Group (TWG) composed of qualified groundwater professionals designated by parties representing more than 80 percent of the total groundwater production from the Indian Wells Valley Groundwater Basin (Basin) was formed to assess groundwater storage in the Basin and evaluate other related technical questions. This paper was the subject of collaboration between these professionals applying scientific methods to estimate the total amount of groundwater and usable groundwater in storage in the Basin. This effort required defining the physical parameters of the Basin, including its geologic and hydrologic characteristics. Three separate methodologies were considered, and the average groundwater volumes estimated from those three approaches are as follows:

1. The total volume of groundwater in storage in the Basin is approximately 66.9 million acre-feet (AF); and
2. The amount of fresh groundwater in storage in the Basin is approximately 37.5 million AF.

2.0 Introduction

2.1 Background

Beneficial users of groundwater in the Basin have a common interest in understanding the amount of groundwater resources available. As such, a TWG has been formed to evaluate these issues and work collaboratively to estimate the total amount of groundwater and usable groundwater in storage. The TWG consists of technical representatives of beneficial users of groundwater that constitute over 80 percent of the pumping in the Basin. The TWG parties include the Indian Wells Valley Water District (District) represented by Krieger & Stewart Engineering Consultants (K&S), Parker Groundwater, and Ramboll, Meadowbrook Dairy represented by Luhdorff & Scalmanini Consulting Engineers (LSCE), Mojave Pistachios represented by aquilogic, Inc., and Searles Valley Minerals Inc. represented by Geoscience Support Services, Inc. (Geoscience).

This paper presents an overview of the collective work performed to date by members of the TWG related to the evaluation of groundwater storage volumes within the Basin (**Figure 1**).

2.2 Terms and Definitions

The following defined terms will be used throughout this paper:

- “Unconfined aquifer” (or water-table aquifer) is defined as “an aquifer in which the water table forms the upper boundary” (Freeze and Cherry, 1979, p. 48);
- “Confined aquifer” is defined as “an aquifer that is confined between two aquitards. In a confined aquifer, the water level in a well usually rises above the top of the aquifer” (Freeze and Cherry, 1979, p. 48); and
- “Aquitard” is defined as “the less-permeable beds in a stratigraphic sequence. These beds may be permeable enough to transmit water in quantities that are significant in the study of regional groundwater flow, but their permeability is not sufficient to allow the completion of production wells within them” (Freeze and Cherry, 1979, p. 47).

The following definitions are used throughout this paper to differentiate total groundwater storage from other subsets of groundwater in storage that are contained within that total volume:

- Total Storage – the total quantity of water in the zone of saturation within a groundwater basin;
- Total Fresh Water in Storage – the quantity of water in the zone of saturation with a total dissolved solids (TDS) concentration of less than 1,000 mg/L; and
- Total Brackish / Saline Water in Storage – the quantity of water in the zone of saturation with a TDS concentration of greater than or equal to 1,000 mg/L.

2.2.1 Definition of Storage

The California Department of Water Resources (DWR) defines “Groundwater in Storage” as “the quantity of water in the zone of saturation.” (Bulletin 118 Definition as described in Best Management Practices – Water Budget [DWR, 2016]). Furthermore, DWR defines “Groundwater Storage Capacity” as “the volume

of void space that can be occupied by water in a given volume of a formation, aquifer, or groundwater basin.” (DWR, 2016).

Freeze and Cherry (1979), defined “storativity” (S) as “the volume of water that an aquifer releases from storage per unit surface area of the aquifer per unit decline in the component of hydraulic head normal to that surface” (p. 60). Storativity, also referred to as the older term “storage coefficient”, describes the capacity of an aquifer to store or release water.

For an unconfined aquifer, the storativity (S) represents the total volume of water that drains by gravity-induced flow from the saturated aquifer. In this case, storativity (S) can be several orders of magnitude larger than for a confined aquifer. For unconfined aquifers, the amount of water stored due to the compressibility of water and the aquifer geologic matrix is negligible, and storativity (S) is called specific yield (Sy), which is expressed as a decimal fraction of 1 or a percentage (%).

Freeze and Cherry (1979), defined the “Specific Storage (Ss) of a saturated aquifer” as “the volume of water that a unit volume of aquifer releases from storage under a unit decline in hydraulic head” (p. 58).

For a confined aquifer, the storativity (S) is equal to the specific storage (Ss) times the aquifer thickness (b), and considers both the compressibility of the aquifer geologic matrix, and the compressibility of water. The storativity (S) can be determined from constant rate aquifer tests that include observation wells. In confined aquifers, storativities typically range from 0.005 to 0.00005 (Freeze and Cherry, 1979, p. 60).

For **unconfined aquifers**, the total volume of groundwater in storage is the total saturated volume multiplied by the Sy:

$$\text{Storage} = (\text{total saturated volume}) \times (\text{Sy}) \dots\dots\dots (\text{Eqn. 1})$$

Where:

Storage	=	Total volume of groundwater in storage [AF]
Total saturated volume	=	Volume of material saturated with groundwater [AF]
Sy	=	Specific Yield [unitless]

For **confined aquifers**, the total volume of groundwater in storage is the total volume of the confined aquifer multiplied by the storativity (S) plus the total saturated volume of the confined aquifer multiplied by the Sy:

$$\text{Storage} = [(\text{total volume}) \times S] + [(\text{total saturated volume}) \times \text{Sy}] \dots\dots\dots (\text{Eqn. 2})$$

Where:

Storage	=	Total volume of groundwater in storage [AF]
Total volume	=	Total volume of the confined aquifer [AF]
S	=	Storativity [unitless]
Total saturated volume	=	Volume of material saturated with groundwater [AF]
Sy	=	Specific Yield [unitless]

For confined aquifers, once the head in a confined aquifer is reduced below the top of the aquifer, unconfined conditions exist. In this case, to calculate the total groundwater in storage in a confined

aquifer, both the volume of stored water under pressure, and the volume of unconfined water must be determined. The first part of the equation represents the total groundwater in storage while the aquifer is confined. The second part of the equation represents the storage when the aquifer becomes unconfined.

The ultimate development potential of a groundwater basin is constrained by several factors. Some of these factors, such as the economic and institutional ones, can change with time. Other factors, however, present significant physical and chemical constraints that will continue to limit the potential for groundwater development. Both types of factors can play a role. Some of these main factors include the following:

- **Physical.** The basin recharge area may not be adequate to sustain development. The pumping could be too concentrated in a portion of basin. Well yields may be too low for the intended use, or the desired pumping rates;
- **Quality.** The water quality may not be suitable for the intended use without treatment. In coastal areas, there is an increased potential for seawater intrusion. Upwelling of poorer quality water in deeper parts of a basin can occur in some instances;
- **Economic.** There can be excessive costs associated with increased pump lifts, and the deepening of wells. There can be high costs associated with treating water if it does meet the requirements for its intended use;
- **Environmental.** Groundwater development may be constrained by the need to maintain groundwater levels for wetlands, stream baseflow, or other groundwater dependent ecosystems (GDE); and
- **Institutional.** Local groundwater management plans or ordinances may be in place that restrict use. Other factors include basin adjudication or impacts on surface water rights.

The volume estimates detailed below follow the three definitions described above for Total Storage, Total Fresh Water in Storage, and Total Brackish / Saline Water in Storage, which can be considered usable groundwater. However, the amount of recoverable groundwater in storage is limited by the factors listed above, including potential undesirable impacts. A determination of recoverable water was not made as part of this study.

2.3 Basin Setting

The Indian Wells Valley Basin (Basin) is a large, alluvium-filled groundwater basin in the Mojave Desert region of Southern California (**Figure 1**). According to DWR Bulletin 118: California's Groundwater (DWR, 2016), the Basin covers an area of 382,000 acres (597 square miles). The Basin lies with an active tectonic area of California and is bordered by and contains numerous active faults that have contributed to its geometry. The Basin consists of an asymmetric structural basin that is deeper on the west, with two areas (El Paso and China Lake) separated by a subsurface bedrock high, which has been filled with thousands of feet of alluvial sediments eroded from surrounding mountains consisting largely of igneous and metamorphic rocks. Interfingering with the alluvial deposits extending out from the mountains that rim the Basin are playa lake and lacustrine deposits found to be more than 1,000 feet thick in the China Lake

Area. Beneath the lacustrine deposits, several thousand feet of coarse-grained sediments that thicken to the west and thin to the east extend down to the bottom of the alluvial basin (**Figure 2**).

2.3.1 Hydrostratigraphy

The Berenbrock and Martin (1991) study, focused on the China Lake Area of the Basin, conceptualized this area of the Basin as a two-aquifer system (**Figure 3**). The shallow aquifer consists of a mixture of some older lacustrine deposits, shallow alluvium underlain by lacustrine deposits, younger lacustrine deposits, playa deposits, and sand dune deposits. The base of the shallow aquifer was poorly defined but assumed to slope from the west side of the China Lake Area, at an elevation of 1,950 feet above mean sea level (amsl), to the east and beneath China Lake, at an elevation of 1,850 feet amsl. The water-bearing deposits in the shallow aquifer primarily consist of fine sand, silt, and clay. In the eastern part of the Basin, lower permeability lacustrine and playa deposits confine, or partly confine, the underlying aquifer.

The deep aquifer includes the alluvium and lacustrine deposits in areas where the shallow aquifer is absent, and the alluvium and lacustrine deposits underlying the shallow aquifer in the eastern part of the Basin (**Figure 4**). The base of the deep aquifer is the bottom of the alluvium, and the saturated thickness of the deep aquifer was estimated to be at least 1,000 feet (Kunkel and Chase, 1969). The deep aquifer was assumed to be unconfined in most places, except the eastern part of the valley where it is confined by lacustrine and playa deposits consisting of silts and clays. The deep aquifer consists of medium to coarse sands with some gravels. The deep aquifer is the main water source for the Basin (Berenbrock and Martin, 1991).

The United States Bureau of Reclamation (USBR) (1993) identified an 800-foot- to 1,300-foot-thick clay layer extending over the majority of the China Lake Area of the Basin and underlying the shallow aquifer system. Sand and fine gravel valley fill the southwestern portion of the Basin and along the Sierra Nevada mountain front. The USBR (1993) showed that the lower permeability sediments extended further west toward the Sierra Nevada than previous conceptualizations had indicated (**Figure 5**), leading to a refinement of the two-aquifer system toward a more complex three-aquifer system.

TetraTech EMI (2003) reviewed data from nearly 300 wells in the China Lake Area of the Basin to create maps, cross-sections, and geochemical plots to identify three discrete hydrogeologic water-bearing zones. They designated these three zones as the Shallow Hydrogeologic Zone (SHZ), the Intermediate Hydrogeologic Zone (IHZ), and the Deep Hydrogeologic Zone (DHZ) (**Figure 6**). There is an extensive Pleistocene lake-deposited clay in the northern portion of the China Lake Area of the Basin that thins and tapers out to the south. This three-zone conceptualization continued in the later work of TriEco TetraTech (2012) (**Figures 7, 8, and 9**).

In recent years as the Hydrogeological Conceptual Framework (HCF) has evolved, these three zones are now more commonly referred to as Hydrogeologic Zones, or HGZs. HGZ1 is the former SHZ. HGZ2 is the former IHZ, and HGZ3 is the former DHZ.

In consideration of the three water bearing zones, there are two groundwater production units in the China Lake Area of the Basin: the saturated portion of HGZ1, and the regional aquifer comprising the saturated portions of HGZ2 and HGZ3.

Groundwater within HGZ1 is generally limited to the eastern and northern portions of the China Lake Area of the Basin, where it occurs under unconfined or perched conditions on top of the low-permeability

lacustrine clays of the upper portion of the HGZ2. Where present, these clays generally act as a barrier between HGZ1 and HGZ2/HGZ3. The depth to groundwater in HGZ1 is generally shallowest in the eastern portion of the China Lake Area of the Basin near the City of Ridgecrest sewage treatment ponds, ranging between 5 feet and 10 feet bgs.

The regional aquifer (saturated portion of HGZ2 and HGZ3) is primarily composed of fan deposits of sands and gravels with some interbedded lacustrine clays. Groundwater within the regional aquifer may occur under confined, semi-confined, or unconfined conditions. Where the lacustrine clays are present, groundwater is semi-confined to confined. Groundwater conditions become unconfined where these clays pinch out. In general, the regional aquifer is unconfined in the vicinity of Inyokern and in the western and southernmost portions of the City of Ridgecrest. In the eastern portion of the Basin, the regional aquifer is confined or semi-confined by lenses of the lacustrine and playa deposits.

Groundwater levels measured in wells screened in the regional aquifer are shallowest in the vicinity of the City of Ridgecrest sewage treatment ponds, where depths to water ranged from 22 feet to 34 feet bgs in two wells with screened intervals from 353 feet to 395 feet bgs. Groundwater levels are deepest south of Inyokern Road and east of Jacks Ranch Road, with depths to water ranging from 220 feet to over 350 feet bgs.

In developing a groundwater model for the Basin, Brown and Caldwell (2009) further refined the TetraTech EMI (2003) Conceptual Hydrologic Model (CHM), and included a large portion of the El Paso Area. Brown and Caldwell discarded the HGZ nomenclature, instead calling the hydrogeologic zones “layers”. Additionally, they parsed one of the HGZs to include an additional layer (**Figure 10**). Thus, the Brown and Caldwell groundwater model included the following four distinct layers:

- Layer 1 – Playa, lacustrine and eolian, alluvial silt and clay deposits as an unconfined aquifer;
- Layer 2 – Unconsolidated young alluvium, playa/lacustrine, and alluvial fan deposits as an unconfined / confined aquifer with variable transmissivity;
- Layer 3 – Older alluvium, more consolidated alluvial fan and basin fill deposits as an unconfined / confined aquifer with variable transmissivity; and
- Layer 4 – Older continental basin fill, heavily cemented, low permeability deposits of the Goler and Ricardo Formations as an unconfined / confined aquifer with variable transmissivity.

Additionally, within the Brown and Caldwell CHM, the following four distinct hydrostratigraphic features were identified (**Figure 10**):

- Fines plug – located in the western part of the Basin between Highway 395 and the Little Lake Fault;
- Gravel zone – located in the southwestern portion of the base between Highway 395 and the City of Ridgecrest;
- Playa – located on the eastern side of the Basin to the east of the Little Lake Fault; and
- High gradient zone – located to the southwest separating the El Paso Area from the China Lake Area of the Basin.

The Desert Research Institute (DRI) (McGraw et al., 2016), under contract to the United States Navy (USN), developed updates to the Brown and Caldwell (2009) model. Specifically, DRI incorporated revised estimates of playa evaporation rates and mountain front recharge while increasing the grid resolution of the model in both the vertical and horizontal directions. Additionally, DRI refined the model layering, by adding two additional layers, to better represent the aquifer units. This change increased the number of layers in the model from four to six (**Figure 11**). An additional layer was added to the unconsolidated, younger alluvium (Brown and Caldwell Layer 2), and one was added to the older basin fill (Brown and Caldwell Layer 4). The purpose of this refinement was to allow greater material property heterogeneity in the vertical direction because the simulated water levels were sensitive to how the clay lenses were vertically distributed. Additionally, the two new layers allowed for better estimates of average pore velocities (McGraw et al., 2016). **Figure 11** shows the general linkages between the various nomenclatures that have been used to describe the three-aquifer system. The DRI “Shallow” zone is effectively HGZ1, and is represented as DRI model layer 1. The DRI “Intermediate” zone is effectively HGZ2, and is represented as DRI model layers 2 and 3. The DRI “Deep” zone is effectively HGZ3, and is represented as DRI model layers 4, 5, and 6.

DRI updated its model in 2017 to incorporate regional faults as groundwater barriers in order to improve predictions of water levels in the El Paso Valley (DRI, 2017). Additional data processing was also incorporated into the updated model to remove duplicate and erroneous data.

Ramboll (2024), has reinterpreted the existing seismic survey data for the Basin, and the revised conceptualization of the Hydrogeologic Conceptual Model (HCM) includes a deeper, fourth HGZ as illustrated on **Figure 12**.

2.3.2 Basin Hydraulic Properties

Numerous investigators have estimated hydraulic parameters within the Basin, and generally nearly all of these are from the China Lake Area. Analysis methods used to estimate hydraulic properties have included reviewing geologic logs from various studies, drillers logs from water wells drilled throughout the Basin, aquifer tests, specific capacity tests, and literature values from studying Basin and Range lithologies (Kunkel and Chase, 1969; Dutcher and Moyle, 1973; USBR, 1993; Anderson et al., 1992; Schwartz and Zhang, 2003). Based on available historical information, Brown and Caldwell (2009) developed a range of hydraulic properties (hydraulic conductivity [K], horizontal to vertical hydraulic conductivity ratio [Kx/Kz], specific storage [Ss], and specific yield [Sy]) for the following four layers in its model, from shallowest to deepest:

- Playa and lacustrine deposits (Layer 1) – K values ranging from 0.1 feet per day (ft/d) to 100 ft/d, a Kx/Kz ratio of 10, and Sy values ranging from 0.05 to 0.15;
- Younger, unconsolidated alluvium (Layer 2) – K values ranging from 0.1 ft/d to 75 ft/d, a Kx/Kz ratio of 10, Ss values ranging from 0.00001 per foot (ft⁻¹) to 0.0001 ft⁻¹, and Sy values ranging from 0.05 to 0.12;
- Older alluvium (Layer 3) – K values ranging from 0.1 ft/d to 75 ft/d, a Kx/Kz ratio of 10, Ss values ranging from 0.00001 ft⁻¹ to 0.0001 ft⁻¹, and Sy values ranging from 0.05 to 0.15; and
- Older basin fill (Layer 4) – K values ranging from 0.1 ft/d to 50 ft/d, a Kx/Kz ratio of 10, and Ss values ranging from 0.00001 ft⁻¹ to 0.0001 ft⁻¹.

DRI subsequently utilized a pilot-point methodology (Doherty, 2003) within the Parameter Estimation (PEST) software module to automate the steady-state calibration process to develop a heterogeneous hydraulic conductivity field for their model. Measured hydraulic conductivity values were used in the pilot-point schema as fixed values, while other hypothetical values were added in areas without measurements. Hydraulic conductivity values for all model cells were determined by interpolating between the measured and hypothetical values.

The DRI interpretation showed areas of higher hydraulic conductivity in the western central part of the China Lake Area of the Basin, with isolated pockets of lower conductivity zones to the northwest, east, southeast, and southwest. A zone of higher hydraulic conductivity is assumed in the western part of the Basin within the shallow layers of the model (Layers 1, 2, and 3), with hydraulic conductivities decreasing to the east. Layers 4 and 5 within the model have uniformly lower hydraulic conductivities extending west to east across the Basin. Layer 6 is limited to the western part of the Basin and has higher hydraulic conductivities.

Based on the interpretation of DRI, specific yields in the main groundwater production areas of the Basin (both the agricultural areas on the western part and the municipal production areas in the southern portion of the China Lake Area of the Basin) are approximately 0.15. The remaining areas of the Basin including the playa and regions to the northeast and southwest in the El Paso Area have specific yields that are approximately 0.25.

In the updated model (DRI, 2017), horizontal hydraulic conductivity distribution in the upper three layers of the model was further refined. Specific yield (S_y) and specific storage (S_s) were also refined but information on those resulting distributions was not provided by DRI in their 2017 technical memorandum (DRI, 2017).

In comparing the distribution of hydraulic conductivities from 2016 to 2017, the area of higher hydraulic conductivities now extends north to south throughout the central and northeastern parts of the China Lake Area of the Basin, while still extending toward the western boundary. The isolated pockets of lower hydraulic conductivity zones, while still located to the northwest, east, southeast, and southwest, have been reduced in size, and the resulting intermediate hydraulic conductivity zones have filled in the areas between the higher and lower hydraulic conductivity zones. Future refinements to the Basin model are expected to show additional changes in the distribution of hydraulic parameters.

3.0 Groundwater Storage Volume – Estimate 1

The TWG developed an estimate of the total volume of groundwater in the Basin utilizing data from the DRI groundwater model documentation (DRI, 2018; 2019a; 2019b; McGraw et al., 2016). The TWG relied on cross-sections and data extracts in published and generally available presentation documents provided by DRI at various times during the past several years. Using those documents from the DRI model documentation, the TWG “built” a three-dimensional Basin volumetric model with the following key features and assumptions:

1. DRI model boundary (lateral extent) (DRI, 2018; McGraw et al., 2016), although it should be noted that the DRI model boundary does not cover the entire Basin;
2. Kern County Water Agency (KCWA) groundwater levels from 2015 (the most recent data available at the time the model was built);
3. Five DRI vertical cross-sections to estimate the shape of the Basin and the DRI model layers (DRI, 2019b; McGraw et al., 2016);
4. DRI TDS concentration distributions in the shallow, intermediate, and deep layers of the DRI model (DRI, 2019a; McGraw et al., 2016); and
5. DRI Sy distributions within the lateral extent of the model boundary (DRI assumed no vertical variation) (DRI, 2019b; McGraw et al., 2016).

3.1 Methodology

Based upon the assumption made by DRI that Sy does not vary vertically and that, over most of the Basin, the Sy is nearly uniformly high at 0.225, it appears that DRI has assumed that the majority of the Basin within the modeling domain is an unconfined aquifer.

The calculation of the total storage of groundwater (volumetric model) in the Basin model area was accomplished using the general equation:

$$\text{Storage} = (\text{total saturated volume}) \times (\text{Sy}) \dots\dots\dots (\text{Eqn. 1})$$

Where:

Storage	=	Total volume of groundwater in storage [AF]
Total saturated volume	=	Volume of material saturated with groundwater [AF]
Sy	=	Specific Yield [unitless]

Complexity in the DRI model resulted from the following:

1. Vertical variation in groundwater quality by partitioning of six stacked layers within the DRI model with disparate lateral variations, divided by hydrogeologic boundaries;
2. Spatial variation in Sy; and
3. Lateral variation in groundwater quality.

The primary software used for calculating the volume was Surfer 21.2.192 (Surfer™) (Surfer). Surfer is software primarily utilized for visualizing geological, hydrological, and environmental data. Surfaces were created as GRD (grid) files by calculating grids in Surfer using XYZ data and the default kriging gridding

method with point kriging type. The grid consisted of 589 rows (399.66-foot node spacing) and 501 columns (400-foot node spacing). Lateral boundaries were defined using geographic information system (GIS) shapefiles, which were projected in State Plane California Zone V (U.S. feet). The unit of X, Y, and Z data was U.S. feet. The lateral grid extents were between 6500000 and 6700000 (X) and 2300000 and 2535000 (Y) (see **Figure 13**).

The total volume of groundwater in storage in the Basin was laterally constrained to the part of the Basin within the DRI groundwater model boundary (McGraw et al., 2016). This boundary was treated as vertical for the entire model. Three zones were evaluated within this boundary (see **Figure 11** for definitions of the zones and model layers).

The highest (shallowest) zone (HGZ1 consisting of DRI model layer 1) was vertically constrained by a top boundary surface defined by a spring 2015 water table surface (the most recent data available at the time the model was built) and a bottom boundary surface. The grid file for the spring 2015 groundwater surface elevation was calculated using XYZ data from KCWA (see **Figure 14**). The grid file for the bottom boundary surface of HGZ1 (consisting of DRI model layer 1) was calculated in Surfer by extrapolating elevation data measured from three vertical cross-sections from DRI (see **Figure 15** for overview, **Figure 16** for cross-section A-A', **Figure 17** for cross-section B-B', **Figure 18** for cross-section C-C', and **Figure 19** for cross-sections D-D' and E-E').

The middle (intermediate) zone (HGZ2 consisting of DRI model layer 2-3) was vertically constrained in the transect locations by calculating grid file boundary surfaces on top and bottom using the same cross-sections and method for the bottom of DRI model layer 1. Grids were used to extrapolate the topography of the top and bottom of DRI model layers 2 and 3 in three dimensions.

The bottom (deep) zone (HGZ3 consisting of DRI model layers 4, 5, and 6) was vertically constrained by calculating grid file boundary surfaces on top and bottom using the same cross-sections and method for the bottom of DRI model layer 1, but additionally with the measured elevations of the bottom of the additional cross-sections (see **Figure 19**).

These topographic extrapolations produced several small areas wherein the underlying and overlying layers were overlapped (i.e., the underlying layer is above the overlying layer). This is an artifact of extrapolation where empirical data is lacking. In the model's northwestern area, the bottom layer of the model protrudes through the extrapolated topographies of the otherwise overlying layers. To correct for this, pseudo-elevation data were incorporated into the XYZ data that were used to extrapolate the topography of the overlying layers, slightly lifting them up in these areas above the layers they overlaid. This resulted in slight protrusion over the overlying layers through their overlying layers (layers 2 and 3), requiring pseudo data to slightly lift these layers in these small areas as well. Similarly, a small protrusion of the model base through layer 5 in the central northern area required a slight topographic raise of layer 5. Finally, in the central southern part of the model along the border, the extrapolated bottom of layer 1 is slightly above the 2015 groundwater surface, so a pseudo datum was added to the bottom of layer 1 to lower it. The locations of pseudo data are shown as black dots encircled in red in the bottom left of **Figure 20**.

The large apparent "peak" in **Figure 20** results from the increase in elevations of that layer in the cross-sections as shown on **Figure 17** for cross-section B-B', and **Figure 18** for cross-section C-C'. The vertical to

horizontal exaggeration for **Figure 19** is approximately 25 to 1, making the “peak” appear much larger than it is in reality.

Within each of the three zones, the volume calculations reflect variations in S_y . The spatial distribution of S_y values does not vary across zones vertically. The variation in S_y is presented with color gradients ranging from red ($S_y < 0.09$) to blue ($S_y > 0.23$) (see **Figure 21**). This variation was simplified in the volume model by partitioning the model into five S_y value zones represented by red ($S_y = 0.09$), yellow ($S_y = 0.125$), green ($S_y = 0.16$), aqua ($S_y = 0.2$), and blue ($S_y = 0.225$). The S_y figure from the DRI model (DRI, 2018; McGraw et al., 2016) was georeferenced in ESRI™ ArcMap and shapefiles for the S_y divisions were subsequently digitized. The boundaries between these five S_y categories were treated as sharp (non-gradational) vertical boundaries.

The spatial variation in water quality (TDS concentrations) in all three zones was derived from the DRI model documentation (see **Figure 22**) (DRI, 2019a; McGraw et al, 2016). The three DRI model zones have their own unique water quality distributions. Although the original water quality figures divided TDS concentrations into four categories (< 499 mg/L, 500 to 999 mg/L, 1,000 to 4,999 mg/L, and > 5,000 mg/L), in this volumetric model the layers were laterally partitioned between fresh groundwater (TDS < 1,000 mg/L) and brackish / saline water (TDS \geq 1,000 mg/L). Within each layer, the fresh/saline groundwater boundaries were treated as vertical.

The overlap of S_y and TDS divisions creates multiple polygons (polygons AA through BS) within the model that represent unique combinations of the two parameters within each layer. Although there were multiple areas within the same zones with the same unique S_y – TDS combination (e.g., with $S_y = 0.225$ and fresh water), they were partitioned into separate polygons if they were not in contact. For each model layer, a “thickness” layer was calculated by subtracting the bottom elevation grid layer from the top elevation grid layer. The polygons with unique S_y and TDS combinations were then used to isolate the respective parts of the “thickness” layers by blanking data outside of each polygon for each respective part of the layer. Polygons AA through AS were used for layer 1, AT through BD for layers 2 and 3, and BJ through BS for layers 4, 5, and 6. The volume was then calculated in Surfer for each piece of each “thickness” layer. In total, DRI model layer 1 contained 19 unique polygons, DRI model layers 2 and 3 contained 15 separate polygons, and DRI model layers 4, 5, and 6 contained 10 separate polygons. The calculated volume results were in cubic feet and were converted to acre-feet (AF) using the equation [volume in AF] = [volume in cubic feet] x $[2.29568 \times 10^{-5} \text{ AF/cubic foot}]$ (see **Figure 23** for example from Layer 1).

3.2 Results

Figure 23 summarizes the volumes of fresh and brackish / saline water in Layer 1 (DRI model layer 1, also referred to as the “shallow zone” or HGZ1). **Figures 24** and **25** summarize the volumes of fresh and brackish / saline water in Layers 2 and 3, respectively (DRI model layers 2 and 3, also referred to as the “intermediate zone” or HGZ2). **Figures 26, 27, and 28** summarize the volumes of fresh and brackish / saline water in Layers 4, 5, and 6, respectively (DRI model layers 4, 5, and 6, also referred to as the “deep zone” or HGZ3). **Table 1**, shown below, summarizes the volumes of fresh water and brackish / saline water for each of the DRI model zones. The values in **Table 1** have been rounded to the nearest 10,000 AF.

Table 1. Estimates of Volumes of Fresh, Brackish / Saline, and Total Water in Storage within the DRI Model Domain

DRI Model Zone	DRI Model Layer	Volume of Fresh Water in Storage (AF)	Volume of Brackish / Saline Water in Storage (AF)	Total Volume of Water in Storage (AF)
Shallow (HGZ1)	Layer 1	10,970,000	5,810,000	16,780,000
Intermediate (HGZ2)	<i>Layer 2</i>	<i>3,170,000</i>	<i>5,080,000</i>	<i>8,250,000</i>
	<i>Layer 3</i>	<i>3,160,000</i>	<i>5,080,000</i>	<i>8,240,000</i>
	Layer 2 + 3	6,330,000	10,160,000	16,490,000
Deep (HGZ3)	<i>Layer 4</i>	<i>6,290,000</i>	<i>7,780,000</i>	<i>14,070,000</i>
	<i>Layer 5</i>	<i>6,320,000</i>	<i>7,720,000</i>	<i>14,040,000</i>
	<i>Layer 6</i>	<i>12,060,000</i>	<i>19,980,000</i>	<i>32,050,000</i>
	Layer 4 + 5 + 6	24,670,000	35,480,000	60,160,000
Total	All 6 Layers	41,970,000	51,450,000	93,430,000

3.3 Additional Considerations

This method incorporated lateral and vertical aquifer limits and Sy values from the DRI model. However, the lateral and vertical limits of the aquifers within the DRI model do not include all water-saturated sediments within the Basin. Thus, any calculation of total groundwater in storage using such DRI limits would be an under-estimate. In general, the Sy values in the DRI model are extremely high compared to prior estimates (e.g., Kunkel and Chase, 1969) and the groundwater flow model for the Basin developed by Brown and Caldwell (2009). That is, the Sy values in the DRI model are not entirely representative of actual hydrologic conditions in the Basin and might be overestimated by 30 percent, as discussed in later methodologies, and then compared below.

4.0 Groundwater Storage Volume – Estimate 2

The TWG developed a second estimate of the total volume of groundwater (including both fresh and saline / brackish) in the Basin utilizing data from the following sources:

1. Department of Water Resources (DWR, 2022a) Basin boundary (lateral extent);
2. Hydrogeologic unit lateral and vertical extents developed by Ramboll as part of the Hydrogeologic Conceptual Framework (HCF) for IWV (Ramboll, 2019);
3. Groundwater levels from spring 2017 California Statewide Groundwater Elevation Monitoring (CASGEM) Network (DWR, 2022b); and
4. Sy distributions from several published sources including:
 - o Kunkel and Chase (1969);
 - o Johnson (1967); and
 - o Heath (1983).

4.1 Methodology

Figure 29 shows the lateral extent of the Basin with additional area polygons identifying the extent of what is described as the Meadowbrook Dairy, and the U.S. Navy’s de-designated groundwater zone (an area within the Basin that does not qualify for municipal or domestic beneficial use [TriEco TetraTech, 2012, p. ES-3]).

Surface areas for each thickness interval were determined by georeferencing figures from Ramboll’s HCF report (Ramboll, 2019), and then creating GIS shapefiles to calculate the area of each thickness interval. **Figure 30** shows the extent and thicknesses of the hydrogeologic units in Hydrogeological Zone 1 (HGZ1). HGZ1 would roughly be comparable to Layer 1 (shallow zone) in the DRI model. In this model, the HGZ1 area included the El Paso Area. **Figure 31** shows the extent and thicknesses of the hydrogeologic units in HGZ2. HGZ2 would roughly be comparable to Layers 2 and 3 (intermediate zone) in the DRI model. **Figure 32** shows the extent and thicknesses of the hydrogeologic units in HGZ3. HGZ3 would roughly be comparable to Layers 4, 5, and 6 (deep zone) in the DRI model.

For HGZ1, the average depth to water level within each of the thickness intervals was determined using the Spring 2017 CASGEM data. The average depth to water was subtracted from the high- and low-end thickness intervals. Sy values within this interval were estimated from Kunkel and Chase (1969) ranging from a low of 0.09 to a high of 0.13.

For HGZ2, the entire unit was assumed to be saturated. Sy values were determined based on the general lithologic descriptions included in Ramboll (2019). HGZ2 was described as finer lacustrine sediments, primarily clays and silts, with interbedded sands and gravels. Johnson (1967) reported average Sy values of 0.02 for clays, and 0.08 for silts.

For HGZ3, the entire unit was assumed to be saturated. Specific yield values were determined based on Heath (1983) which reported a Sy value of 0.22 for sand and 0.19 for gravel.

4.2 Results

On the basis of the unit thicknesses, their associated areas, and the range of Sy values for each HGZ, estimates for the volumes of water in each layer within each HGZ were prepared. **Tables 2, 3, and 4** below summarize the estimated volumes of water within HGZ1, HGZ2, and HGZ3, respectively. The values in **Tables 2, 3, and 4** have been rounded to the nearest 10,000 AF.

Table 2. Estimates of Volume of Water within HGZ1

HGZ1 Thickness Intervals (feet)			Saturated Thickness (feet)		Area (acres)	Specific Yield (Sy)		Storage (AF)	
Low	High	Average DTW (feet bgs)	Low	High	Area (acres)	Low	High	Low	High
328	500	224	104	276	100,000	0.09	0.13	940,000	3,590,000
164	328	143	21	185	100,000	0.09	0.13	190,000	2,410,000
82	164	93	0	71	66,000	0.09	0.13	0	610,000
0	82	122	0	0	17,000	0.09	0.13	0	0
Total					283,000			1,130,000	6,610,000

Notes: DTW – depth to water; bgs – below ground surface

Table 3. Estimates of Volume of Water within HGZ2

Aquifer Thickness (feet)	Area (acres)	Specific Yield (Low)	Specific Yield (High)	Storage-- Low (AF)	Storage-- High (AF)
>984	61,000	0.02	0.08	1,200,000	4,800,000
656 – 984	54,000	0.02	0.08	710,000	4,250,000
328 – 656	50,000	0.02	0.08	330,000	2,620,000
0 – 328	32,000	0.02	0.08	0	840,000
Total	197,000	0.02	0.08	2,240,000	12,510,000

Table 4. Estimates of Volume of Water within HGZ3

Aquifer Thickness (feet)	Area (acres)	Specific Yield (Low)	Specific Yield (High)	Storage – Low (AF)	Storage – High (AF)
>984	95,000	0.19	0.22	17,760,000	20,570,000
656 – 984	57,000	0.19	0.22	7,100,000	12,340,000
328 – 656	30,000	0.19	0.22	1,870,000	4,330,000
0 – 328	14,000	0.19	0.22	0	1,010,000
Total	196,000	0.19	0.22	26,730,000	38,250,000

Table 5 summarizes the estimated range of water volumes within each HGZ and the total volume within the Basin.

Table 5. Estimates of Volume of Water within Each HGZ and the Basin

HGZ	Area (acres)	Storage – Low (AF)	Storage – High (AF)
1	283,000	1,130,000	6,610,000
2	197,000	2,240,000	12,510,000
3	196,000	26,730,000	38,250,000
Total	676,000	30,100,000	57,370,000

4.3 Additional Considerations

The method described above only calculated groundwater in storage for the China Lake Area of the Basin and did not include estimates for the El Paso Area (except HGZ1). Thus, the actual total groundwater in storage in the entire Basin is greater than the figures shown in **Table 5**.

5.0 Groundwater Storage Volume – Estimate 3

The TWG developed a third estimate of the volumes of groundwater in storage (including fresh, brackish / saline, and total) using the updated Ramboll HCF (2024). The steps taken to develop the groundwater volume estimates using this methodology were as follows:

1. Update the HCF including depths and extents of each layer that formed a unique HGZ representing the lithologies in the Basin;
2. Estimate areas and volumes associated with each HGZ in the Basin;
3. Estimate the percentage net sand in each of the HGZs;
4. Estimate the total volume of material in each HGZ, including net sand volumes and mixed/fines volumes;
5. Tabulate a range of S_y values for clay, mixed sand and clay, fine sand, medium sand, and sand and gravel;
6. Estimate the total volume of groundwater in storage in each HGZ on the basis of the minimum and maximum values for the S_y ranges; and
7. Using existing water quality data for the Basin, estimate the volumes of groundwater in each HGZ that would be considered fresh, and brackish / saline.

5.1 Methodology

5.1.1 Updating the Hydrogeological Conceptual Framework (HCF)

The calculation of storage in the Basin for Estimate 3 is based upon the three-dimensional (3D) HCF model geometry and HGZs produced for the Brackish Groundwater Resources Feasibility Study (Ramboll, 2019), as modified based on the following discussion.

For the hydrogeological conceptual framework model (HCF), four different hydrogeologic zones (HGZs) were mapped. HGZ1 is predominantly unconsolidated sand and gravel with interbedded thinner clayey layers, and is considered to be unconfined. HGZ2, which lies below HGZ1, consists predominantly of unconsolidated clayey sediments, with interbedded productive sands and gravels, and is generally considered to be an aquitard. HGZ3 consists predominantly of unconsolidated sand and gravel, situated underneath HGZ2, though like HGZ1 can have clayey layers interbedded with the sand and gravel deposits as well as mixed lithology. This aquifer is confined where HGZ2 is present, but is unconfined where HGZ1 directly overlies HGZ3. HGZ4 consists of the semi-consolidated to consolidated fluvial, lacustrine, and volcanic rocks of the Ricardo Group, and predominantly alluvial gravel, sand, and clay of the Goler Formation.

Since the development of the hydrogeological conceptual framework (HCF) for the basin in 2019 (Ramboll, 2019), new data have become available. Specifically, parts or all of 12 seismic lines have been reprocessed by Collier Geophysics (Collier Geophysics, 2021 and 2023), transforming time to depth using sonic logs, identifying faults, the Basin bedrock bottom (Basement), and where possible, the surfaces of the HGZs. In addition, well completion reports and one newly installed monitoring well (EP-1) with lithology descriptions and a geophysical log have been added to the database.

The reprocessed seismic lines used in the update of the HCF are shown on **Figure 33**. These seismic lines provide more detailed information on the Basin Basement, as well as the top of HGZ3 and HGZ4. The interpretation of the seismic sections is based upon the few wells that extended into the Basement (Snort 1, Snort 2 and TGCH 1) and correlated between the lines. In addition, the results Monastero et al. (2002) were used in the interpretation of the top of HGZ4 and Basement.

The El Paso Area had the greatest amount of modification during the update. The reprocessed seismic sections showed that the basement in this part of the Basin was much deeper on the west side than previously modeled. A better constraint on the boundary between HGZ3 and HGZ4 was also obtained from the reprocessed seismic data interpretation.

Figure 34 shows the elevation of the bottom of the basement. **Figure 35** contains two cross-sections illustrating the updated HCF. Cross-section A-A' crosses the entire basin from the southwest to the northeast. On this cross-section, the high basement separating the China Lake and the El Paso Areas is easily seen. In addition, A-A' shows how HGZ2 thins and disappears to the northeast of the basement high, then thickening again towards the center of the basin around the playa lake. Cross-section B-B' in **Figure 35** shows the HCF from west to east in the China Lake Area, where HGZ2 is not present in the western part of the basin, though quickly thickening towards the center of the basin. HGZ1 is relatively thin in the El Paso Area as well as in the playa lake portion in the center and eastern part of the China Lake Area. **Figure 36** shows the interpreted seismic section 92-02.

The total area and volume for each HGZ is presented in **Table 6**. The geometry and total varying thickness of the Basin is shown on **Figure 37**, and the thicknesses of each HGZ is shown on **Figure 38**. Note that a minimum thickness of 3 feet for each HGZ unit was used in the volume calculations. The 3-foot minimum was applied for both the distribution and volume of each HGZ, providing a more conservative estimate of the area and volume for each HGZ. This conservative approach takes into account the uncertainties associated with the interpolation of the HGZ surface boundaries in the model.

HGZ1 represents the unconfined aquifer in the Basin. To estimate the amount of groundwater storage capacity available, it is necessary to calculate the thickness of the unsaturated zone so that it can be removed from the total volume. This is done using water level measurements recorded in the wells. This data is supplemented with information on the water table as mapped out from the AEM data collected in 2017. The water level measurements from October 2017 were used to produce the water level map since these data correlate with the collection of the AEM data, providing the best geographic coverage across the basin. These two sources are combined and interpolated into a 100-meter square grid covering the entire Basin, providing an elevation for the water table. The unsaturated zone was thereafter removed from the total Basin volume calculations, resulting in calculations for the saturated sediments only, noting that water levels continued to decline resulting in a slight overestimate of total groundwater in storage.

The total area and volume for each HGZ is presented in **Table 6**.

Table 6. Total Area and Thickness of the HGZs

HGZ	Area (acres)	Area For Volume Calculation (acres) ¹	Total Volume (AF) ²
HGZ1 total	294,000	279,000	89,200,000
<i>HGZ1 saturated</i>	<i>213,000</i>	<i>213,000</i>	<i>38,600,000</i>
HGZ2	350,000	293,000	172,500,000
HGZ3	352,000	282,000	332,300,000
HGZ4	383,000	268,000	460,800,000
Total Basin Volume			1,054,800,000
Total Saturated Basin Volume			1,004,200,000

Notes:

1. A minimum thickness of 3 feet for each HGZ unit was used in the volume calculations.
2. Area rounded to the nearest 1,000 acres and volume rounded to nearest 100,000 AF. HGZ1 is split up into total volume and saturated volume.

5.1.2 Determining the Percentage of Net Sand and Net Clay

Net sand and net clay for each hydrogeologic zone is calculated from available detailed well completion report lithologic descriptions and available geophysical logs for HGZ-1 and HGZ-2, and from the reprocessed seismic lines for HGZ-3 and HGZ-4. This information is used to qualitatively assess the appropriate specific yield range for each HGZ. To determine the percentage sand from the lithology logs, the lithologic descriptions are divided into three categories: coarse, mixed sediments, and fine. Coarse sediments include descriptions where sand, gravel or cobble is the descriptor in the lithology logs. Fine sediments include descriptions where clay and silt are the descriptors. Mixed lithology has both coarse and fine sediments in the descriptor. The well lithologic data are supplemented with interpretation of 61 geophysical logs, where the resistivity logs are used to determine where the layers consist of predominantly sand (resistivities over 30 ohm-m). In areas where total dissolved solids (TDS) are above 1,000 mg/L, the resistivity is too greatly influenced by the salinity and not included in the analysis. **Figure 39** shows an example of how the lithology and the resistivity logs are interpreted. In cases where there are different totals between the lithology and resistivity logs, an evaluation is made based upon the quality of the lithology descriptions. In the case shown on **Figure 39**, there is a highly detailed lithologic description from a cored hole which was used to record the total coarse materials in the hydrogeological zones.

For each well analyzed, the percentage net sand and net clay within the hydrogeologic zone is recorded. The net sand values presented include the percentage of the well lithology description that contain only sand and/or gravel. The net clay values include the percentage of the well lithology description that contain only clay and silt. Lithologies with sand or gravel as the primary and silt or clay as a secondary descriptor in the lithology log description are not included in net sand. The reverse is also true for mixed clay.

The percentage net sand and net clay within a hydrogeologic zone vary spatially. To illustrate this, the percentage net sand is interpolated to a one-mile square grid. From this, a map of the thickness of the net sand for each one-mile grid is created for HGZ1 and HGZ2, which most wells penetrate. Maps showing the net sand for HGZ1 and HGZ2 are shown on **Figure 40** and **Figure 41**. Data from HGZ3 and HGZ4 are sparse and insufficient to estimate net sand from the well information, and instead reprocessed seismic data were used.

Figure 40 shows the results of the calculated net sand for HGZ1, presented as an average distributed in a one-mile square grid. Areas without one-mile squares have no well data providing information on the net sand. Net sand in HGZ1 is generally over 40%, with a Basin-wide average of 52%. The observed net sand values vary significantly across the Basin, with a tendency of higher net sand values to the west, closer to the Sierra Nevada Frontal Fault and in the center of the Basin west of Ridgecrest. Note the sparse well data in the El Paso Area and in the northeastern part of the Basin.

Figure 41 shows the results of the calculated net sand for HGZ2. As expected, there is a lower percentage of net sand in HGZ2, with a Basin-wide average of 23%. However, values do vary significantly over short distances. This is illustrated in the area around Ridgecrest where there are adjacent one-mile squares with net sand varying from less than 20% to over 80%. This underscores the variability of the sand lenses within HGZ2 which are very difficult to model in the HCF. Like for HGZ1, there is sparse well data in the El Paso Area and in the northeastern part of the Basin.

Table 7 shows the average percent net sand, mixed lithology and net clay calculated for HGZ1 and HGZ2. These are based solely on the lithology descriptions and borehole logs. Note that for HGZ2, which is dominated by finer sediments, the net clay is lower than the net sand. However, HGZ2 is dominated by mixed lithology, comprising 62% of the total in the basin. Upon closer review of the lithological descriptions in the wells, most of the descriptions with mixed lithology have clay or silt as the primary descriptor (i.e. sandy clay), and thus there is observed a tendency towards finer sediments in mixed lithology.

Table 7. Calculation of the Percent Net Sand, Mixed Lithology and Net Clay for HGZ1 and HGZ2

HGZ	Net Sand (%)	Mixed (%)	Net Clay (%)
HGZ1	54.7	40.6	4.7
HGZ2	22.8	62.0	15.2

Note: There are not enough wells that penetrate HGZ3 and HGZ4 to calculate the volumes for these zones based upon well data

The net sand and net clay calculated from the reprocessed seismic sections (Collier Geophysics, 2021 and 2023) is used to assess the appropriate range of specific yield values for HGZ3 and HGZ4. The technique used in this study is commonly applied in the oil and gas industry to identify potential reservoirs. This analysis mapped the percentage of net sand units along the reinterpreted seismic lines. **Figure 42** provides shows an example from line 92-02, which shows the net sand along the seismic section. **Figure 43** illustrates the net clay along the same section. These results are presented in **Table 8**. The variability between the different seismic lines is not as great as what is observed in HGZ1 and HGZ2. However, it is noted that in HGZ3 where HGZ2 is not present, the net sand content increases. This can be seen in the

averages for lines 92-02 and 00-07, which are higher than the other lines, and cross this zone. It needs to be noted that the seismic lines were collected to capture the deeper sediment, and thus there is poor resolution above approximately 100 mS (the upper 100-150 meters; Collier Geophysics, 2023). Thus, the net sand and net clay for HGZ1 and HGZ2 was not estimated from the seismic sections.

Table 8. Calculated Net Sand in HGZ3 and HGZ4, Averaged for Each Seismic Line (Collier Geophysics, 2021 and 2023)

Line	Net Sand in HGZ3 (%)	Net Sand in HGZ4 (%)	Net Clay in HGZ3 (%)	Net Clay in HGZ4 (%)
82-01	7.3	10.1	52.6	53.1
88-02	9.3	8.6	46.8	49.0
88-08	12.6	9.3	53.6	59.3
88-01	9.9	13.8	63.6	58.2
88-07	14.3	9.5	45.5	52.7
88-05	13.4	15.8	56.5	54.7
88-04	15.1	17.9	56.4	48.4
82-03	11.7	10.6	46.1	45.8
92-01	13.2	13.9	48.4	49.4
00-07	23.3	15.7	26.3	33.7
92-02	18.9	11.1	34.6	37.7
00-06	16.6	4.1	44.5	66.4
Average	13.8	11.7	47.9	50.7

Note: The location of the seismic lines is shown on [Figure 19](#).

Table 9 shows the saturated volumes for each HGZ, divided into calculated net sand, calculated mixed, and fines.

Table 9. Calculation of the Total Volume, Net Sand Volume and Mixed/Fines (Lithology with Mixed with or Only Containing Silt and/or Clay) for Each HGZ

HGZ	Total Volume (AF)	Net Sand Volume (AF)	Mixed Volume (AF)	Net Clay Volume (AF)
HGZ1	38,600,000	21,100,000	15,700,000	1,800,000
HGZ2	172,500,000	39,300,000	107,000,000	26,200,000
HGZ3	332,300,000	45,800,000	127,300,000	159,200,000
HGZ4	460,800,000	53,900,000	173,300,000	233,600,000

Note: The volumes for HGZ1 are saturated volumes only.

5.1.3 Specific Yield and Storativity

There is limited direct empirical data on S_y obtained from previous studies on the Basin, although previous studies do make S_y assumptions based on observations. Thus, there is uncertainty with regards to the S_y that should be used to calculate groundwater in storage. However, there have been studies that have looked at S_y for the different sediment types, where ranges of S_y have been compiled. The United States Geological Survey (USGS) conducted a thorough study of S_y from different sediment types at numerous locations in California (USGS, 1967). The study produced a range of values for the different sediment types, and are shown in **Table 10**. These values correspond well with the values reported from other general studies, including Heath (1983) and Robson (1993). The values also correspond with the values from the Basin, as reported by Kunkel and Chase (1969).

Table 10. Range of Specific Yield from USGS (1967)

Sediment	S_y Minimum	S_y Maximum
Clay	0.01	0.10
Mixed Sand and Clay	0.04	0.12
Fine Sand	0.10	0.32
Medium Sand	0.15	0.32
Sand and Gravel	0.15	0.25

To accommodate for a range in S_y , total groundwater in storage was calculated using a maximum and minimum value. The percentage net sand for each HGZ has been calculated. For the portion of the HGZ that is net sand, the value for S_y used corresponds to sand and gravel. For the portion of the HGZ that is mixed and fines, the values for mixed sand and clay is used. Thus, the S_y used for each HGZ is simply:

$$[(\% \text{ net sand}) \times (S_y \text{ sand and gravel})] + [(\% \text{ mixed and fines}) \times (S_y \text{ mixed sand and clay})] \quad (\text{Eqn. 3})$$

Where:

% net sand	=	percentage of sand in HGZ unit being considered [%]
S _y sand and gravel	=	specific yield of sand and gravel of HGZ unit [unitless]
% mixed and fines	=	percentage of mixed sand and clay in HGZ unit being considered [%]
S _y mixed sand and clay	=	specific yield of mixed sand and clay of HGZ unit [unitless]

These values for HGZ1 and HGZ2 have been placed into the one-mile square grid based upon the net sand calculations and averaged out for HGZ3 and HGZ4. The result is an average S_y that includes the ranges for both net sand and the mixed materials in the HGZs.

Storativity used to calculate the storage in the confined aquifers is calculated on the one-mile grid. A review of the resulting storage coefficient for the one-mile square grid shows a range of between 1x10⁻³ to 1x10⁻⁶, which correspond well with the values estimated by Dutcher and Moyle (1973).

5.2 Results

For the calculations, the Basin has been divided up into one-mile square grids. This is done to provide a spatial distribution of storage volume throughout the Basin. For each one-mile square, there is an average thickness divided up into net sand thickness and mixed/fines thickness. The minimum and maximum values for S_y are based upon the range of S_y for sand and gravel, as presented in **Table 10**. For the mixed and fines, the S_y used was 0.08 for HGZ1 and HGZ3, as the non-net sand sediments in these zones is dominated by mixed lithologies. For HGZ2, the lithological logs indicate a greater predominance of clay, and thus an S_y of 0.06 is used to account for the higher content of clay in the zone. **Table 11** shows the range of S_y for HGZ1, HGZ2, and HGZ3. For HGZ4, the zone is semi-consolidated to consolidated; thus, S_y is set with a range of 0.06 to 0.10 to accommodate for the lower S_y observed in consolidated materials (Heath, 1983).

The totals for the minimum and maximum total groundwater in storage are shown in **Table 12**. Total groundwater in storage is greatest in HGZ1 and HGZ3, as HGZ1 is unconfined and HGZ3 is partially unconfined. HGZ2 and HGZ4 have lower storage volume estimates since both are confined aquifers. The values in **Tables 11**, and **12** have been rounded to the nearest 100,000 AF.

Table 11. Calculated Total Groundwater in Storage for Each HGZ

HGZ	Saturated Volume (AF)	SY Range	Minimum (AF)	Maximum (AF)
HGZ1	38,600,000	0.12 – 0.17	4,600,000	6,600,000
HGZ2	172,500,000	0.08 – 0.10	13,800,000	17,200,000
HGZ3	326,200,000	0.12 - 0.14	39,100,000	45,700,000
Subtotal	537,300,000	0.11 – 0.13	57,500,000	69,500,000
HGZ4	460,800,000	0.06 – 0.10	27,600,000	46,100,000
Total	998,100,000	0.09 – 0.12	85,100,000	115,600,000

The totals are also divided up for the China Lake Area and El Paso Area, shown in **Table 12**.

Table 12. Minimum and Maximum Total Groundwater in Storage Values Divided up by Basin Area

HGZ	Minimum (AF)		Maximum (AF)	
	China Lake	El Paso	China Lake	El Paso
HGZ1	4,300,000	300,000	6,200,000	400,000
HGZ2	10,400,000	3,400,000	13,100,000	4,100,000
HGZ3	32,700,000	6,400,000	38,100,000	7,600,000
<i>Subtotal HGZ1-3</i>	47,400,000	10,100,000	57,400,000	12,100,000
HGZ4	15,900,000	11,700,000	26,500,000	19,600,000
Total	63,300,000	21,800,000	83,900,000	31,700,000

Total groundwater in storage across the Basin is not evenly distributed. Thus, the distribution of the groundwater in storage for each HGZ represented in each one-mile square in the grid has been calculated.

Figure 44 shows the distribution of minimum storage for HGZ1. This shows that there is a larger volume of total groundwater in storage in the center of the Basin, just west of Ridgecrest, as well as an area with larger volume of total groundwater in storage adjacent to the Coso Range in the northern part of the basin. The area of larger groundwater storage volume in the northern part of the basin corresponds with what has been interpreted as a buried channel or delta observed in the AEM data, which appears to contain fresh water (Ramboll, 2019). In contrast, there is less groundwater in storage in the El Paso Area as well as the eastern portion of the China Lake Area. In both areas, HGZ1 is relatively thin, and particularly in the El Paso Area, nearly completely unsaturated.

Figure 45 shows the distribution of the minimum calculated total groundwater in storage for HGZ2. In the northwestern part of the basin continuing in a band towards Ridgecrest, HGZ2 is relatively thin to not

present, and thus total groundwater storage in HGZ2 is limited there. Total groundwater storage in HGZ2 is greatest in the center of the basin.

Figure 46 shows the distribution of the minimum groundwater in storage for HGZ3 and **Figure 47** shows the distribution of the minimum groundwater in storage for HGZ4.

The totals for the median of the total groundwater in storage are also divided up with regards to water quality. The available TDS data was contoured, with the one-mile square grid for each HGZ divided up into the following water quality zones:

- TDS under 1,000 mg/L, representing fresh groundwater resources;
- TDS from 1,000 – 3,000 mg/L, representing transitional groundwater resources; and
- TDS over 3,000 mg/L, representing brackish and saline groundwater resources.

There is no available data for HGZ4 and thus the totals for HGZ3 were used for HGZ4. However, this indicates a high amount of uncertainty associated with the water quality in HGZ4 and the totals should be considered with caution. The groundwater in storage totals with respect to water quality are presented in **Table 13**. The volume of groundwater with TDS values of under 1,000 mg/L across the Basin is shown on **Figure 48**.

The volumes in **Table 13** should not be interpreted as available in their entirety to meet water-supply demands; complete dewatering of any aquifer is environmentally undesirable. The recoverable groundwater in storage is determined on the basis physical, water quality, economics, environmental, and institutional factors (DWR 2003), including the potential for undesirable impacts, and has not yet been determined.

Table 13. Minimum and Maximum Storage Value for Each HGZ, Divided Up Into Water Quality Zone, with Respect to Total Dissolved Solids (TDS) Concentrations

HGZ	Minimum Groundwater in Storage Considering Water Quality (AF)			Maximum Groundwater in Storage Considering Water Quality (AF)		
	Under 1,000 mg/L	1,000-3,000 mg/L	Over 3,000 mg/L	Under 1,000 mg/L	1,000-3,000 mg/L	Over 3,000 mg/L
HGZ1	3,500,000	400,000	700,000	5,000,000	600,000	1,000,000
HGZ2	6,700,000	300,000	6,800,000	8,400,000	300,000	8,500,000
HGZ3	19,700,000	800,000	18,600,000	22,900,000	900,000	21,900,000
<i>Subtotal HGZ1-3</i>	<i>29,900,000</i>	<i>1,500,000</i>	<i>26,100,000</i>	<i>36,300,000</i>	<i>1,800,000</i>	<i>31,400,000</i>
HGZ4	15,200,000	900,000	11,500,000	25,500,000	1,300,000	19,300,000
Total	45,100,000	2,400,000	37,600,000	61,800,000	3,100,000	50,700,000

5.3 Additional Considerations

The storage calculations conducted in Estimate 3 are based upon the revised HCF model, which has been developed using the best available science and most recent data, including the AEM data collected in 2017 and the recently reprocessed seismic lines. Net sand has been calculated from the wells and seismic sections, providing the best available total and net sand volume calculations in the basin, which helped in selecting the range of specific yield values to use for the total groundwater in storage calculations. That said, there is still a significant amount of uncertainty associated with the groundwater storage calculations.

A large amount of the uncertainty in the groundwater storage calculations lies with the limited availability of direct empirical data on specific yield for the sediments in the IWV Basin. There have been only a few specific yield values that have been determined directly through aquifer testing, with none calculated from wells in the El Paso Area of the basin. Because of this uncertainty, a range for specific yield based the UGSG report (USGS 1967) was used. This range is large and resulted in a difference of a factor of 2.4 between the minimum and maximum calculated total groundwater in storage. Empirical data on S_y for the different sediment types in the Basin, calculated from appropriately designed and executed aquifer tests would help refine the values for specific yield.

There is also high uncertainty associated with the water quality in the deeper zones, particularly HGZ4, as well as in the El Paso Area as a whole. Collecting more water quality samples from the deeper zones basin-wide and the central and southern portion of the El Paso Area would be useful to determine if the water quality in these zones is adequate for potable supply.

6.0 Discussion

Table 14 summarizes the key differences between the three groundwater storage estimates including differences in the following:

- Methodology Used;
- Areas Considered;
- Number of HGZs Considered; and
- Volumes Considered.

As a result of these key differences, a direct comparison between all three estimates is not possible. However, some notable and supportable conclusions can be reached regarding the volume estimates.

Table 14. Key Differences Between the Three Groundwater Storage Estimates

Key Difference	Estimate 1	Estimate 2	Estimate 3
Methodology	DRI Model Boundary KCWA 2015 Water Levels DRI Vertical Cross-Sections DRI TDS Concentrations DRI Sy Distributions	DWR Basin Boundary Ramboll HGZ CASGEM 2017 Water Levels Literature Sy	DWR Basin Boundary Revised HGZs (Added HGZ4) Net Sand / Mixed / Fines Literature Sy DRI TDS Concentrations
Areas Considered	China Lake & El Paso Areas	China Lake Area	China Lake & El Paso Areas
Number of HGZ	3	3	4
Volumes Considered	Total, Fresh, Brackish	Total	Total, Fresh, Transitional, Brackish

6.1 Total Groundwater in Storage

Table 15 compares the total volumes of groundwater in storage, noting that Estimate 2 did not consider the volume stored in the El Paso Area, and Estimate 3 included an additional deeper HGZ4 volume that was not included in the other estimates.

Average values can be calculated for the three methods used to estimate total groundwater in storage for HGZ1 through HGZ3, noting that Estimate 2 did not consider the groundwater in the El Paso Area. While there is uncertainty in each of the three groundwater volume estimates, and differences in volumes that resulted from the different methodologies utilized, averaging the three estimates provides a “middle” range that more likely represents actual groundwater volumes within the Basin. Differences between the volume estimates for each of the methodologies is discussed below in **Section 6.3**. The averages for the three methods indicate the following:

- The total groundwater in storage in HGZ1 and HGZ2 is approximately **21,870,000 AF**. This groundwater is readily accessible using existing wells or new wells screened within these zones.

Additional infrastructure (i.e., wells, pumps, pipelines) may be needed to access some of this groundwater; and

- The total groundwater in storage in HGZ1 through HGZ3 is approximately **66,890,000 AF**. This groundwater is accessible but additional infrastructure (i.e., wells, pumps, pipelines) would be needed to access some of this groundwater, notably in HGZ3.

Table 15. Total Groundwater Volumes in Storage by Estimate and HGZ

HGZ	Type	Estimate 1	Estimate 2		Estimate 3		Average of Methods
		Value [AF]	Low [AF]	High [AF]	Low [AF]	High [AF]	Value [AF]
HGZ1	Range	---	1,130,000	6,600,000	4,600,000	6,600,000	---
	Average	16,780,000	3,870,000		5,600,000		8,750,000
HGZ2	Range	---	2,240,000	12,520,000	13,800,000	17,200,000	---
	Average	16,490,000	7,380,000		15,500,000		13,120,000
HGZ3	Range	---	26,740,000	38,240,000	39,100,000	45,700,000	---
	Average	60,160,000	32,490,000		42,400,000		45,020,000
Sub-Total	Range	---	30,110,000	57,370,000	57,500,000	69,500,000	---
	Average	93,430,000	43,740,000		63,500,000		66,890,000
HGZ4	Range	---	---		27,600,000	46,100,000	---
	Average	---	---		36,850,000		---
Total	Range	---	---		85,100,000	115,600,000	---
	Average	---	---		100,350,000		---

Note: Estimate 2 only considered the volume stored in the Main Basin (China Lake) and excluded the El Paso Area.

6.2 Fresh and Brackish / Saline Groundwater in Storage

Table 16 compares the estimated volumes of groundwater in each HGZ that are considered “fresh”, with a TDS concentration of less than 1,000 mg/L, and “brackish / saline”, with a TDS concentration of greater than or equal to 1,000 mg/L.

Average values can be calculated for the two methods used to estimate fresh and brackish/saline water in storage for HGZ1 through HGZ3. The averages for the two methods indicate the following:

- The total fresh groundwater in storage in HGZ1 and HGZ2 is approximately **14,550,000 AF**. This groundwater is readily accessible using existing wells or new wells screened within these zones. Additional infrastructure (i.e., wells, pumps, pipelines) may be needed to access some of this groundwater.

- The total fresh groundwater in storage in HGZ1 through HGZ3 is approximately **37,530,000 AF**. This groundwater is accessible but additional infrastructure (i.e., wells, pumps, pipelines) would be needed to access some of this groundwater, notably in HGZ3.

Table 16. Total Fresh and Brackish / Saline Groundwater Volumes in Storage by Estimate and HGZ

HGZ	Type	Estimate 1		Estimate 3				Average of Methods	
		Fresh [AF]	Brackish [AF]	Fresh [AF]		Brackish [AF]		Fresh [AF]	Brackish [AF]
HGZ1	Range	---	---	3,500,000	5,000,000	1,100,000	1,600,000	---	---
	Average	10,970,000	5,810,000	4,250,000		1,350,000		7,610,000	3,580,000
HGZ2	Range	---	---	6,700,000	8,400,000	7,100,000	8,800,000	---	---
	Average	6,330,000	10,170,000	7,550,000		7,950,000		6,940,000	9,060,000
HGZ3	Range	---	---	19,700,000	22,900,000	19,400,000	22,800,000	---	---
	Average	24,670,000	35,480,000	21,300,000		21,100,000		22,990,000	28,290,000
Sub-Total	Range	---	---	29,900,000	36,300,000	27,600,000	33,200,000	---	---
	Average	41,970,000	51,460,000	33,100,000		30,400,000		37,530,000	40,930,000
HGZ4	Range	---	---	15,200,000	25,500,000	12,400,000	20,600,000	---	---
	Average	---	---	20,350,000		16,500,000		---	---
Total	Range	---	---	45,100,000	61,800,000	40,000,000	53,800,000	---	---
	Average	---	---	53,450,000		46,900,000		---	---

Note: Estimate 2 did not differentiate between fresh groundwater and brackish / saline groundwater. As a result, the average values in this table are based upon Estimate 1 and Estimate 3, and the sum of the fresh and brackish / saline averages do not equal the total averages shown in Table 15 that are based upon all three estimates.

6.3 Groundwater Volume Differences Between the Various Methodologies

Specific yield (Sy) is one of the key drivers for differences between the various volume estimates. For Estimate 1, the assumed Sy in the DRI model is relatively high and likely unrepresentative. A value of approximately 0.225 was utilized across the majority of the model domain. This factor, coupled with vertical homogeneity across all model layers, suggests that Estimate 1 overestimates the volume of groundwater in storage within the Basin. The Sy for Estimate 2 varied from 0.02 to 0.19 in the “Low” scenario, and 0.08 to 0.22 in the “High” scenario. The Sy for Estimate 3 varied from 0.08 to 0.12 in the “Low” scenario, and 0.10 to 0.17 in the “High” scenario. Since Estimate 2 and Estimate 3 uses ranges of Sy values, volume of groundwater in storage is expected to fall somewhere near the middle of the ranges.

Table 17 summarizes the results for when the Estimate 1 methodology is reworked utilizing the Sy ranges from the Estimate 2 and Estimate 3 methodologies. Using the Estimate 2 Sy ranges, within the Estimate 1 model framework the total volume of groundwater ranges between a low of approximately 64,900,000 AF

to a high of approximately 81,690,000 AF. This range is between 69 percent and 87 percent of the baseline Estimate 1 total volume of approximately 93,400,000 AF. Using the Estimate 3 Sy ranges, within the Estimate 1 model framework the total volume of groundwater ranges between a low of approximately 53,100,000 AF to a high of approximately 62,700,000 AF. This range is between 57 percent and 67 percent of the baseline Estimate 1 total volume of approximately 93,400,000 AF.

Table 17. Total Groundwater Storage Volumes Using Estimate 1 Methodology with Different Sy

HGZ	Estimate 1	Using Estimate 2 Sy Ranges		Using Estimate 3 Sy Ranges	
	[AF]	Low [AF]	High [AF]	Low [AF]	High [AF]
HGZ1	16,780,000	7,040,000	10,180,000	9,390,000	13,300,000
HGZ2	16,490,000	1,580,000	6,350,000	8,170,000	7,950,000
HGZ3	60,160,000	56,280,000	65,160,000	35,540,000	41,470,000
Total	93,430,000	64,900,000	81,690,000	53,100,000	62,720,000

A second key driver for differences between the various groundwater volume estimates is differences in lithological assumptions utilized in the three estimate models. **Figure 49** shows a comparison in lithologies for a similar cross-section between Estimate 1 and Estimate 3. While the HGZs in the Estimate 1 cross-section are, for the most part, of uniform thickness along the cross-section, there are significant changes in all three HGZ thicknesses along the cross-section in the Estimate 3 cross-section.

These lithological layer thickness and extent differences, coupled with the Sy differences described above, will result in groundwater volumes that vary (in some cases significantly) between the three methodologies utilized above.

6.4 Estimated Groundwater Volumes Compared To The GSP

The Groundwater Sustainability Plan (GSP) for the Basin (Indian Wells Valley Groundwater Authority [IWVGA], 2020, p. 3-26) refers to and utilizes the 1993 United States Bureau of Reclamation (USBR) estimated groundwater volume of 2,370,000 AF as “available groundwater in storage”. The GSP then estimates the remaining groundwater in storage as of 2017 as 1,750,000 AF. This value is likely a gross underestimation of the remaining groundwater within the Basin for the reasons illustrated above.

Based on the three methodologies described in this paper, the total estimated average volumes of “fresh” groundwater remaining in the Basin are approximately 7,610,000 AF in HGZ1 and 6,940,000 AF in HGZ2, for a combined total of approximately 14,550,000 AF. These volume estimates are 4.3 times larger than the GSP value for HGZ1, 4.0 times larger for HGZ2, and 8.3 times larger for both HGZ1 and HGZ2 combined.

7.0 Conclusions

A TWG of qualified groundwater professionals representing parties that pump more than 80 percent of the groundwater in the Indian Wells Valley Groundwater Basin conducted a series of analyses to estimate the total amount of groundwater and usable groundwater in storage. This effort required defining the physical parameters of the Basin, including its geologic and hydrologic characteristics. Three separate methodologies were considered, and the following conclusions can be drawn from this work as further described below.

1. Regardless of which estimating methodology is used, given the size of the Basin (area and depth of lithologies), the volume of groundwater in storage is large, ranging from a low of approximately **30.1 million AF** (excluding the El Paso Area) to a high of **115.6 million AF** (including HGZ4) (see **Table 15**). Using the average of the three methods approximately **21.9 million AF** of total groundwater in storage is readily accessible in HGZ1 and HGZ2. Under the average of the three methodologies, an additional **45.0 million AF** of total groundwater in storage is available within HGZ3, for a total of **66.9 million AF** available in HGZ1 through HGZ3.
2. There is a substantial volume of fresh water within the Basin ranging from a low of approximately **42.0 million AF** to a high of **61.8 million AF** (see **Table 16**). Using the average of Estimates 1 and 2, approximately **14.5 million AF** of fresh groundwater in storage is readily accessible in HGZ1 and HGZ2 and an additional **23.0 million AF** of fresh groundwater in storage is available within HGZ3, for a total of **37.5 million AF** of fresh water available in HGZ1 through HGZ3.
3. There is a substantial volume of brackish / saline water within the Basin that has the potential to be utilized as a resource subject to treatment to reduce TDS concentrations. These volumes range from a low of approximately **40.0 million AF** to a high of **53.8 million AF** (see **Table 16**).
4. Specific yield (Sy) is one of the key drivers for differences between the various volume estimates. The assumed Sy in the DRI model is a relatively high and likely unrepresentative value of approximately 0.225 across the majority of the model domain. This factor, coupled with vertical homogeneity across all model layers, suggests that the DRI model overestimates the volume of groundwater in storage within the Basin. Accordingly, Estimate 1 using the DRI model assumptions likely overestimates the total groundwater in storage. Estimate 2 did not include groundwater in storage in the El Paso Area. The Estimate 3 methodology estimated groundwater in storage across the entire Basin and adopted a range of more realistic Sy values based on sediment types. Therefore, the estimates using the Estimate 3 methodology are likely most representative of reality and closely match the average values from the three approaches. Specifically, the estimate using the Estimate 3 methodology produced a range of **57.5 to 63.5 million AF** of groundwater in storage in HGZ1 through HGZ3, while the average of the three methods produced an estimate of approximately **66.9 million AF** of groundwater in storage in HGZ1 through HGZ3.

5. Aquifer pumping test data in several areas of the Basin, and conducted on wells screened within the various lithologies, would be the best way to reduce the uncertainty associated with the current variability in S_y .
6. A second key driver for differences between the various groundwater volume estimates is differences in lithological assumptions utilized in the three estimate models (see **Figure 49**). While the HGZs in the Estimate 1 cross-section are, for the most part, of uniform thickness along the cross-section, there are significant changes in all three HGZ thicknesses along the cross-section in the Estimate 3 cross-section.
7. The Estimate 3 methodology is notably rigorous because it involved the calculation of groundwater storage in the Basin based on a three-dimensional (3D) HCF model geometry and HGZs produced for the Brackish Groundwater Resources Feasibility Study. Significant insight was garnered as a result of the updated Ramboll HCF (specifically Basin and HGZ geometry) coupled with the data utilized in the net sand and mixed / fines analysis.
8. The estimates have identified the approximate volumes of groundwater in storage across HGZ1 through HGZ3 or HGZ1 through HGZ4, depending on the methodology. The recoverable groundwater in storage would be determined on the basis of physical, water quality, economic, environmental, and institutional factors including the potential for undesirable impacts.

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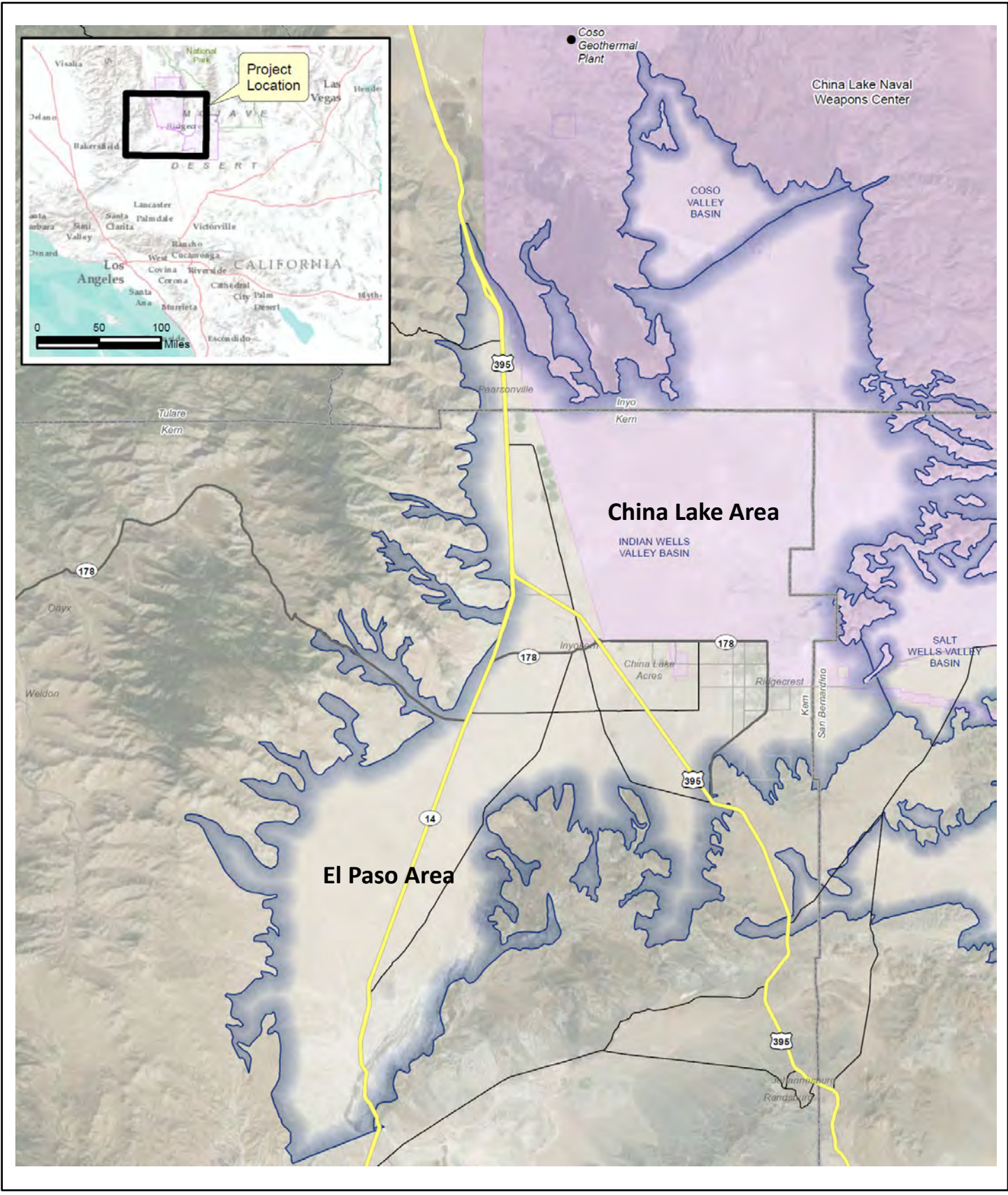
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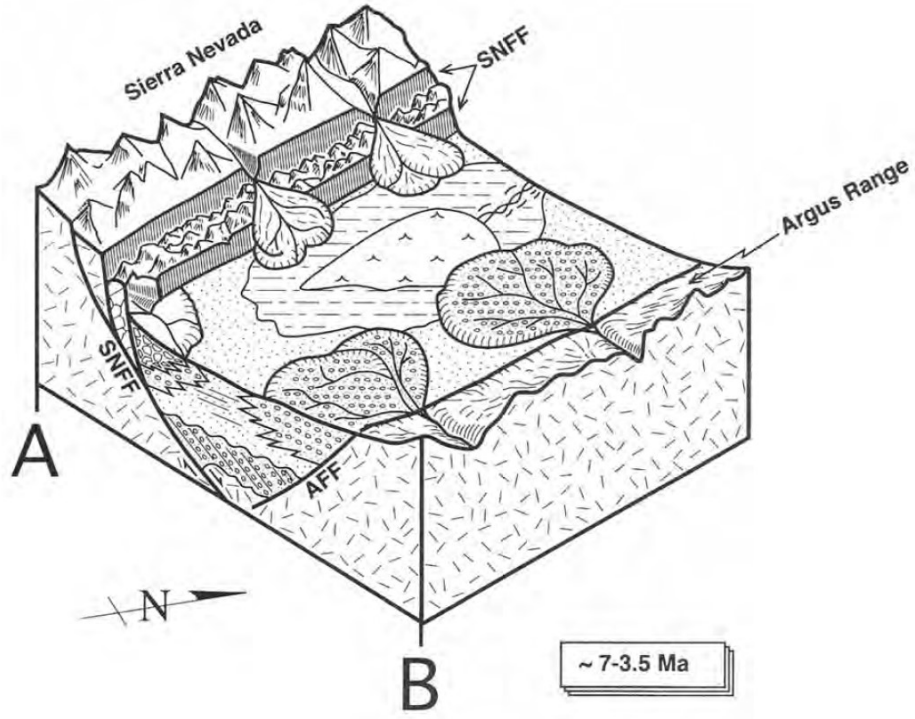
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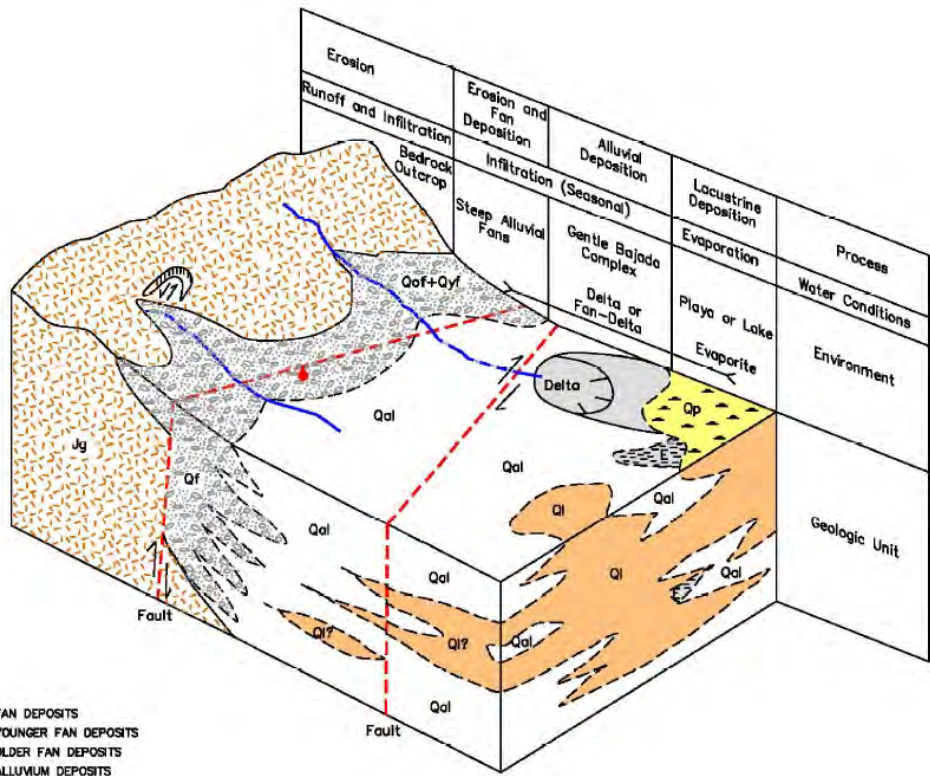
Figures



		Indian Wells Valley Technical Working Group Indian Wells Valley Area Map
Date: 9/18/2023		Figure 1



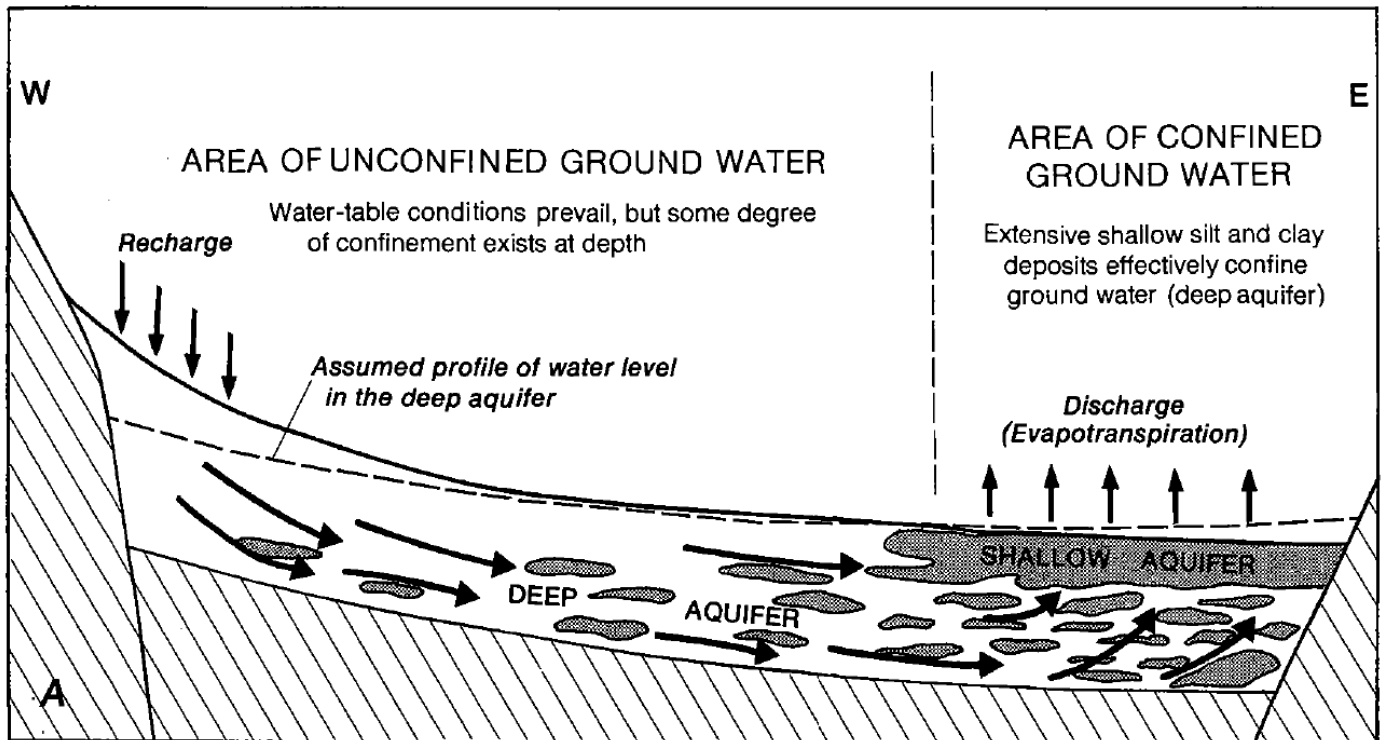
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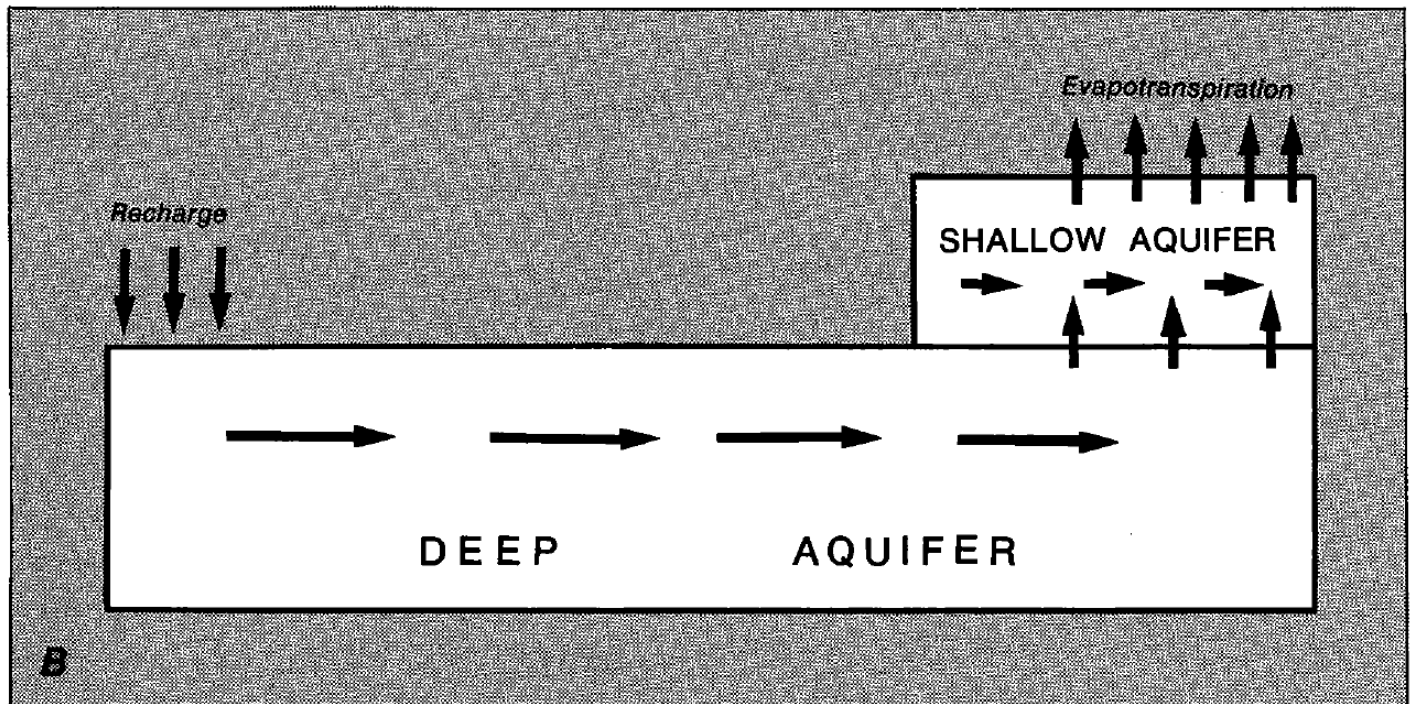
- Notes:
- Qf QUATERNARY FAN DEPOSITS
 - Qyf QUATERNARY YOUNGER FAN DEPOSITS
 - Qof QUATERNARY OLDER FAN DEPOSITS
 - Qal QUATERNARY ALLUVIUM DEPOSITS
 - Ql QUATERNARY LACUSTRINE DEPOSITS
 - Qp QUATERNARY PLAYA DEPOSITS
 - Jg JURASSIC GRANDDIOIRITE
 - E EVAPORITE

Reference: TetraTech EMI, 2003. Groundwater Management in the Indian Wells Valley Basin, Ridgecrest, California. AB 303 Grant. State of California Water Resources Department, dated June.

FINE-GRAINED UNCONSOLIDATED DEPOSITS
 COARSE-GRAINED UNCONSOLIDATED DEPOSITS
 CONSOLIDATED ROCKS



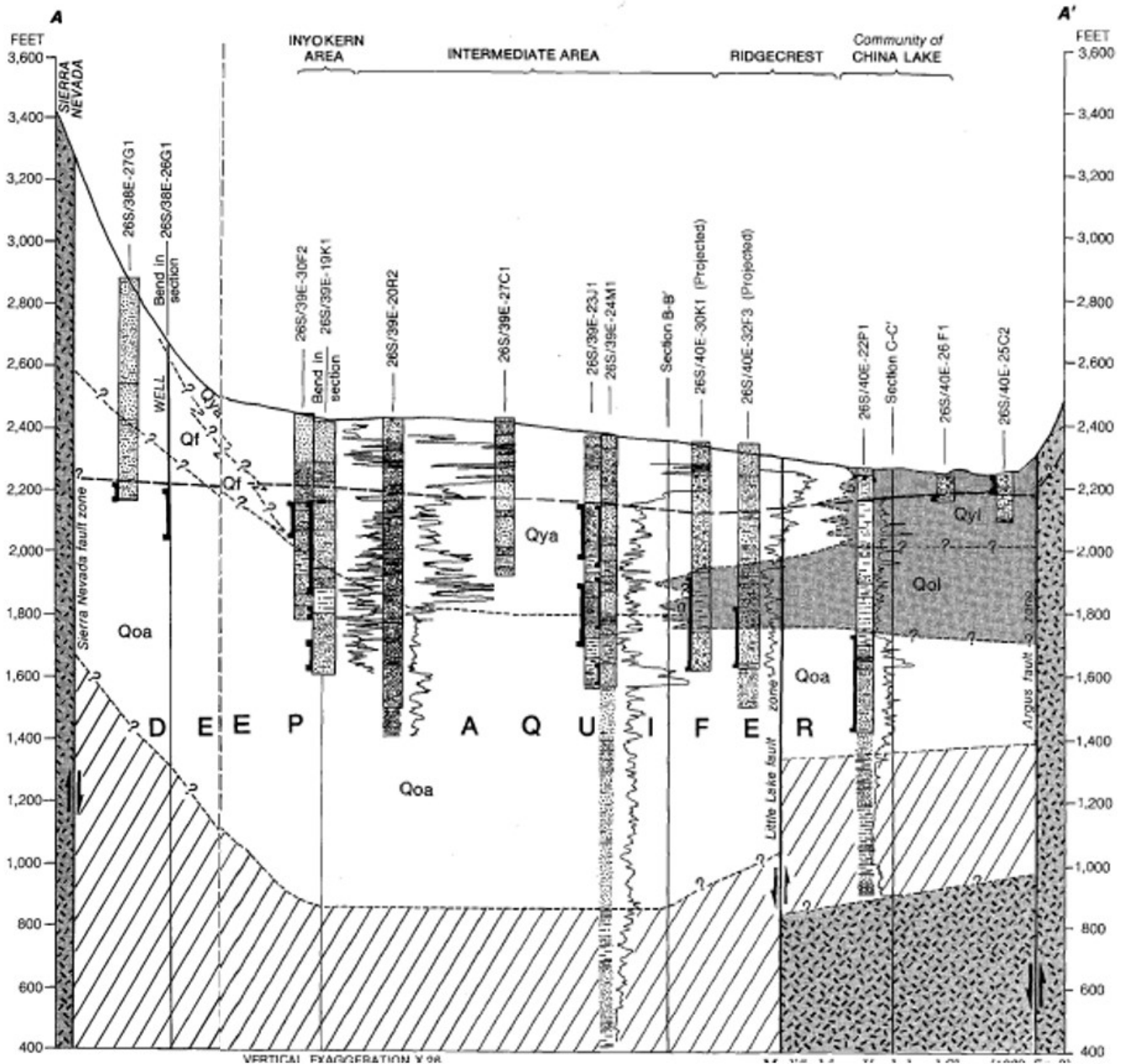
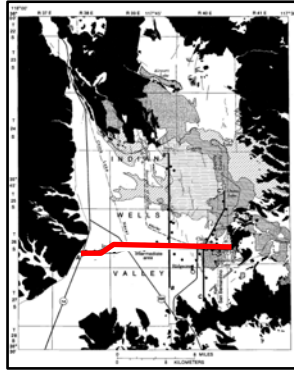
(Modified from Warner, 1975, p. 10)



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Indian Wells Valley
 Technical Working Group
 Indian Wells Valley
2-Aquifer Conceptualization

Date: 9/18/2023 **Figure 3**



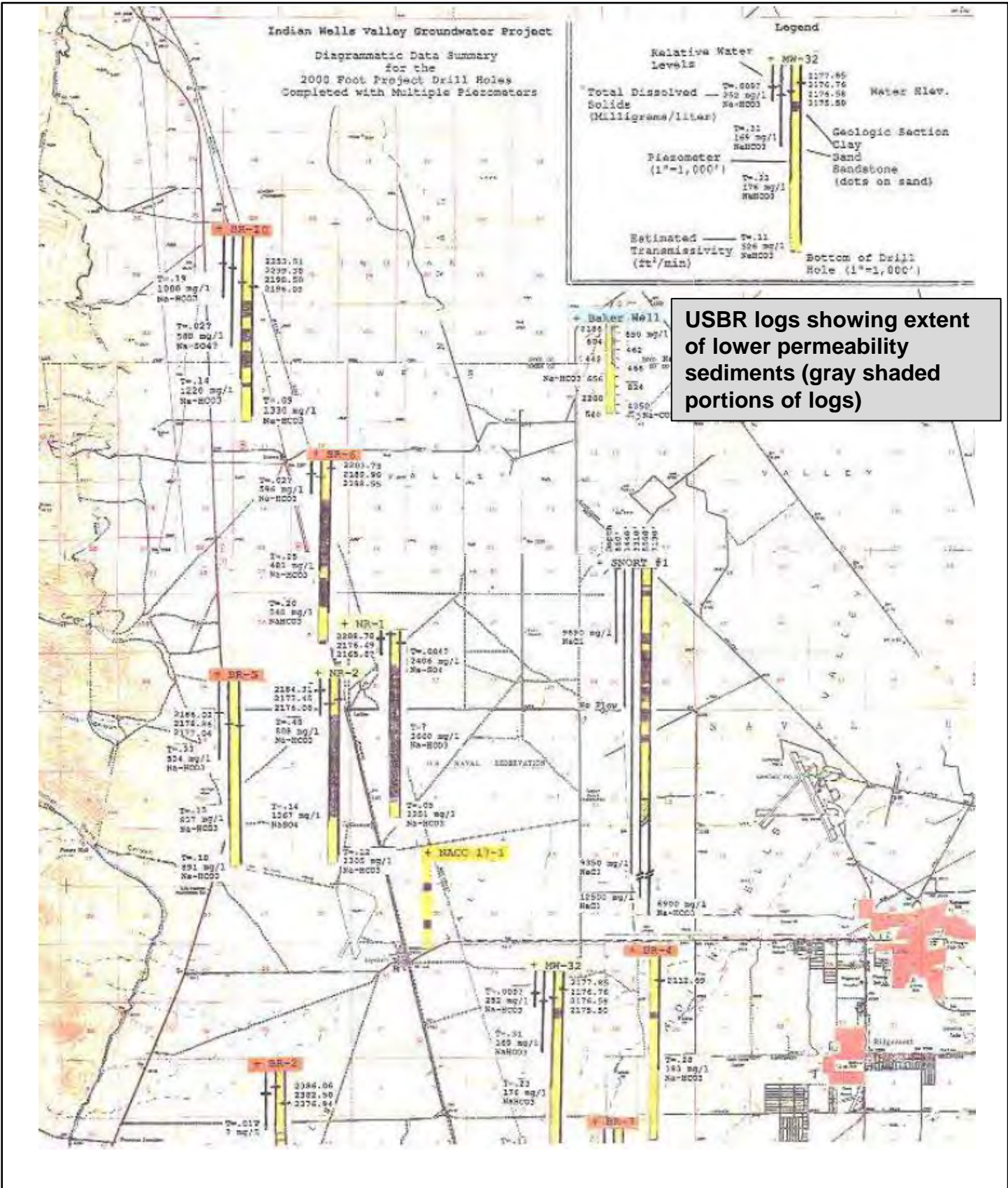
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Indian Wells Valley
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Indian Wells Valley
2-Aquifer West-East Cross-Section

Date: 9/18/2023

Figure 4

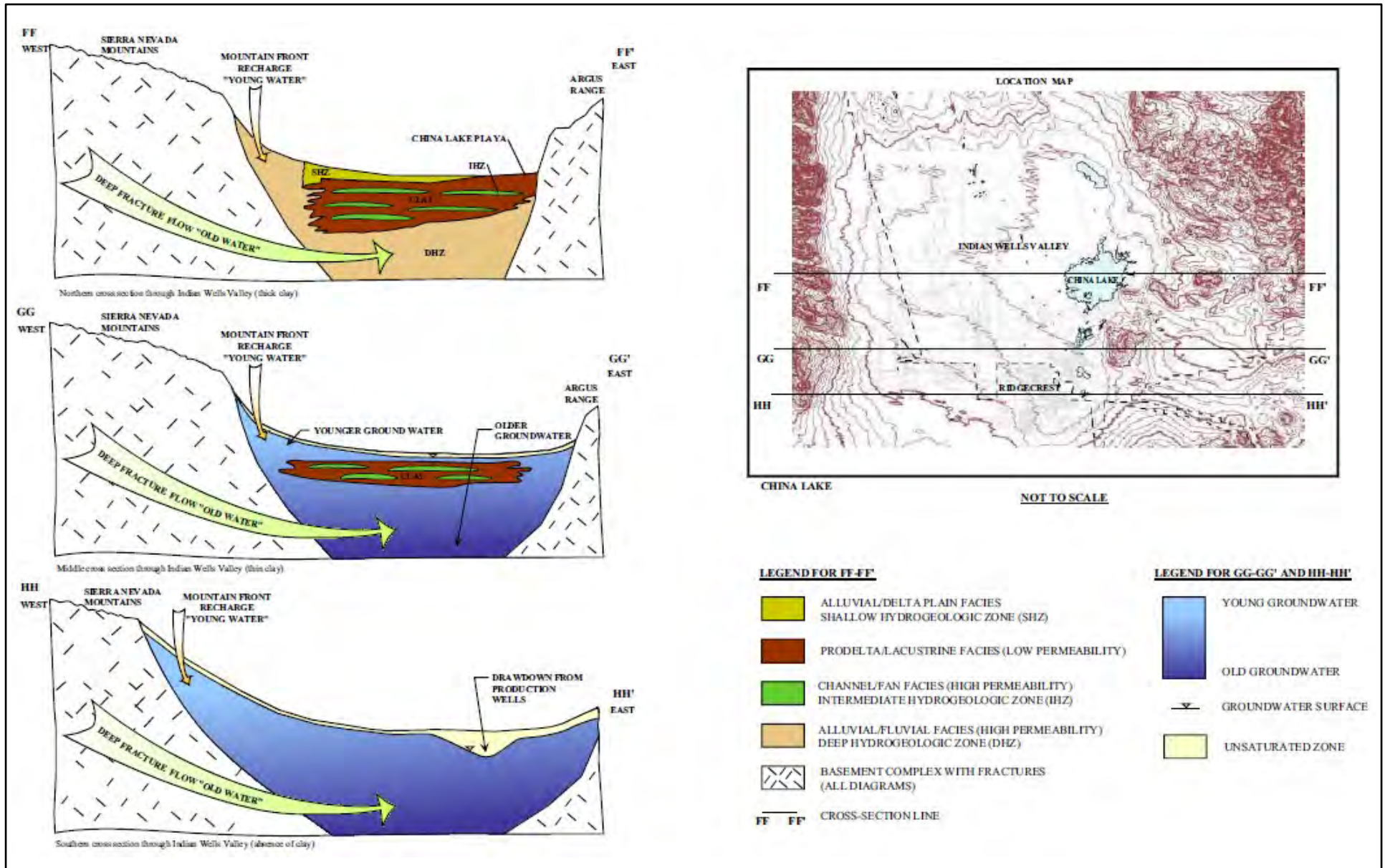


Reference: United States Bureau of Reclamation (USBR), 1993. Indian Wells Valley Groundwater Project. Volume II Technical Report, dated December.

Indian Wells Valley
 Technical Working Group
 Indian Wells Valley
**USBR Logs Showing Lower
 Permeability Sediments**

Date: 9/18/2023

Figure 5



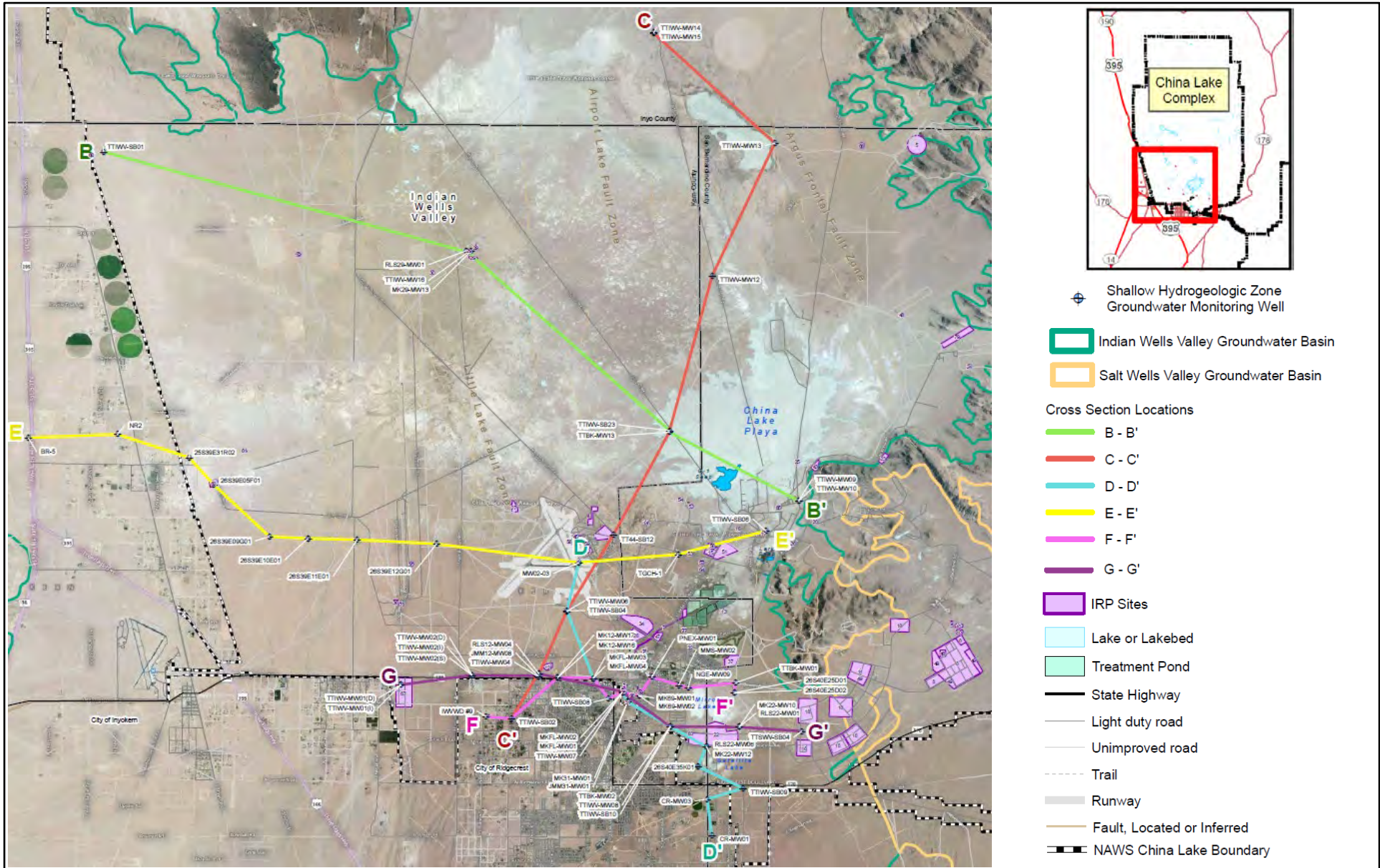
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Indian Wells Valley
3-Aquifer Conceptualization

Date: 9/18/2023

Figure 6



Reference: TriEco TetraTech, 2012. Technical Justification for Beneficial Use Changes for Groundwater in Salt Wells Valley and Shallow Groundwater in Eastern Indian Wells Valley. Naval Air Weapons Station China Lake, California. TRIE-2205-0004-0003, dated May 25.

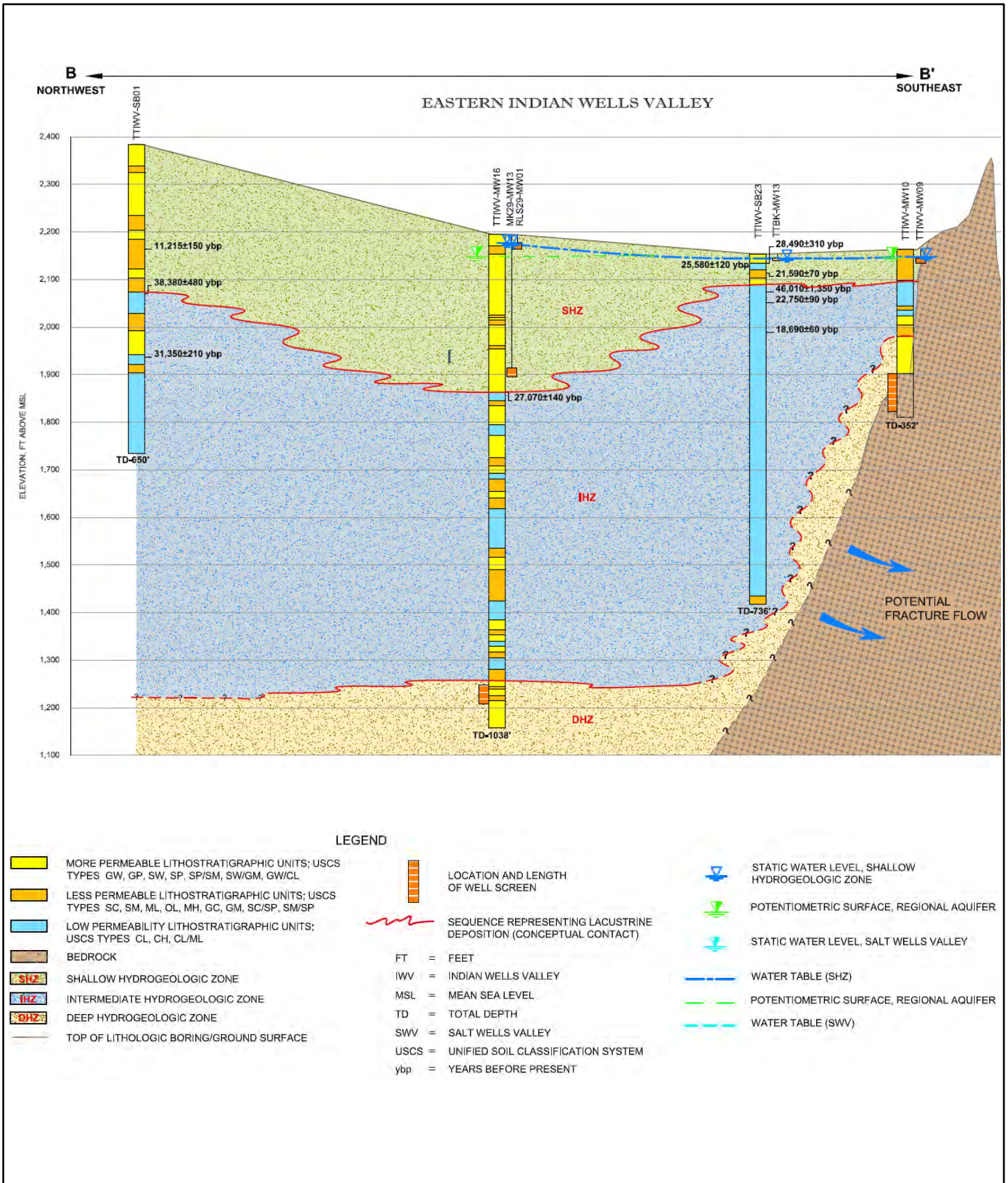
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Indian Wells Valley

Recent Basin Cross-Sections

Date: 9/18/2023

Figure 7

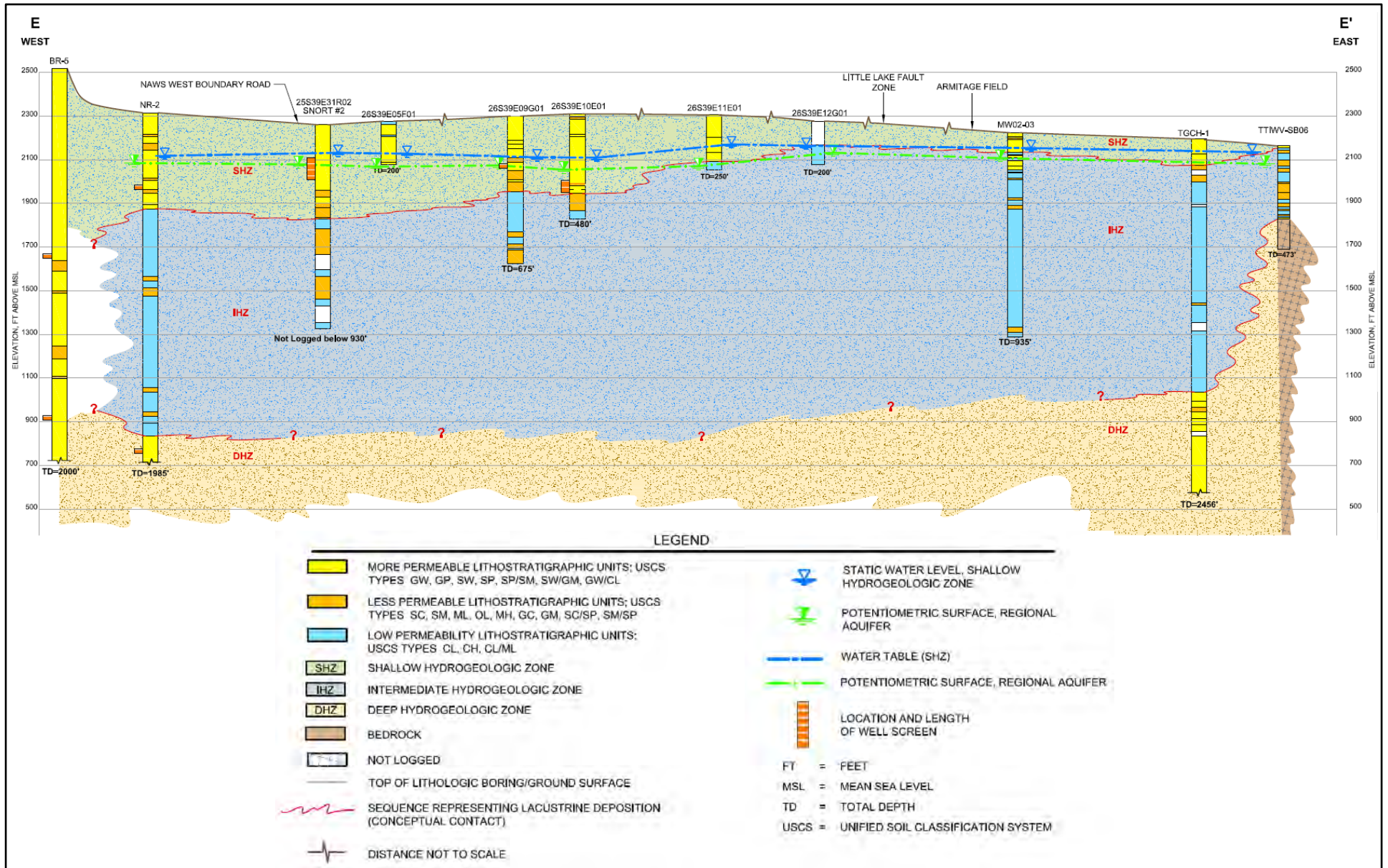


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Indian Wells Valley
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Indian Wells Valley
Basin Cross-Section B-B'

Date: 9/18/2023		Figure 8
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Reference: TriEco TetraTech, 2012. Technical Justification for Beneficial Use Changes for Groundwater in Salt Wells Valley and Shallow Groundwater in Eastern Indian Wells Valley. Naval Air Weapons Station China Lake, California. TRIE-2205-0004-0003, dated May 25.

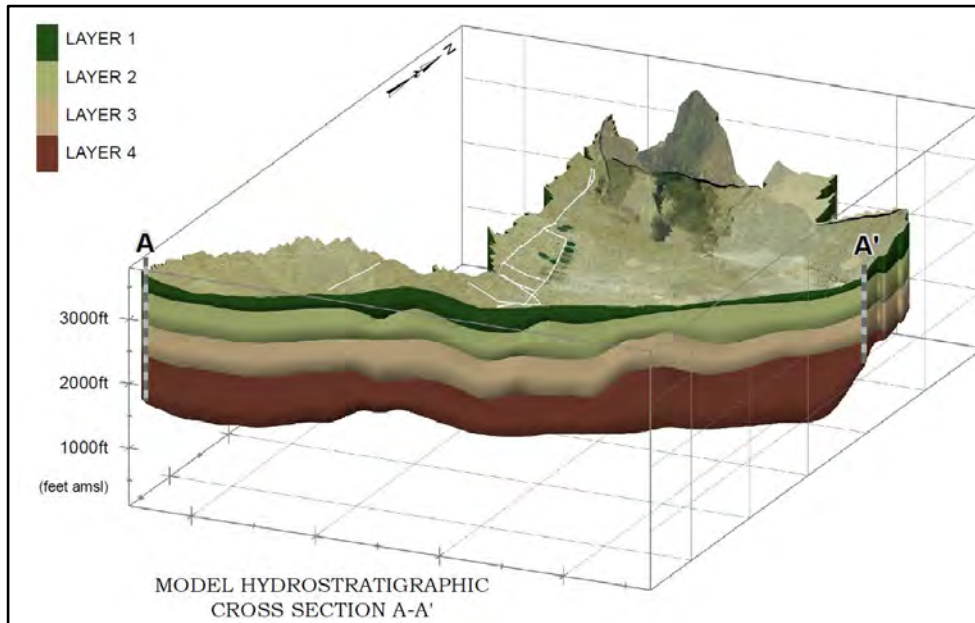
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Indian Wells Valley

Basin Cross-Section E-E'

Date: 9/18/2023

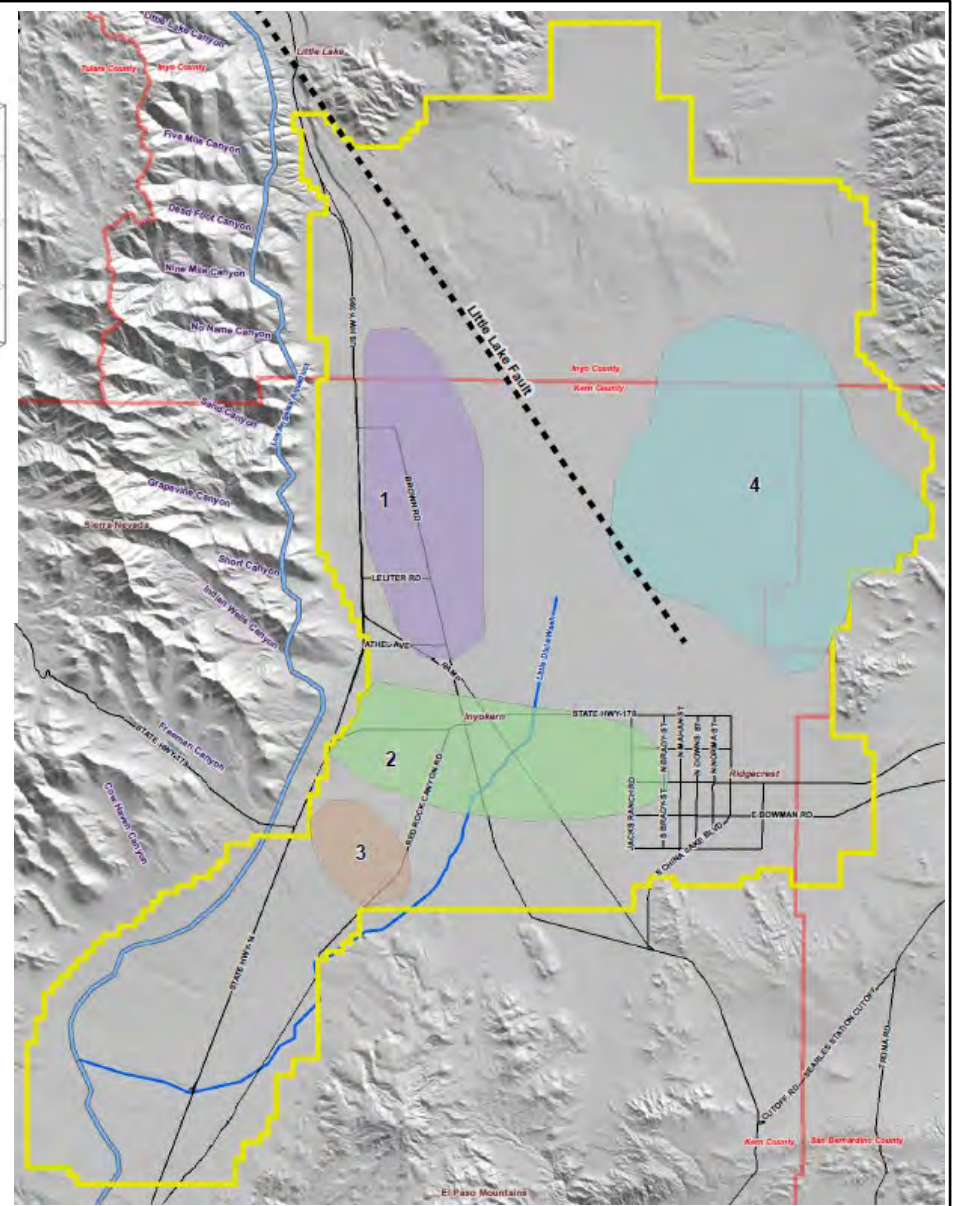
Figure 9



EXPLANATION

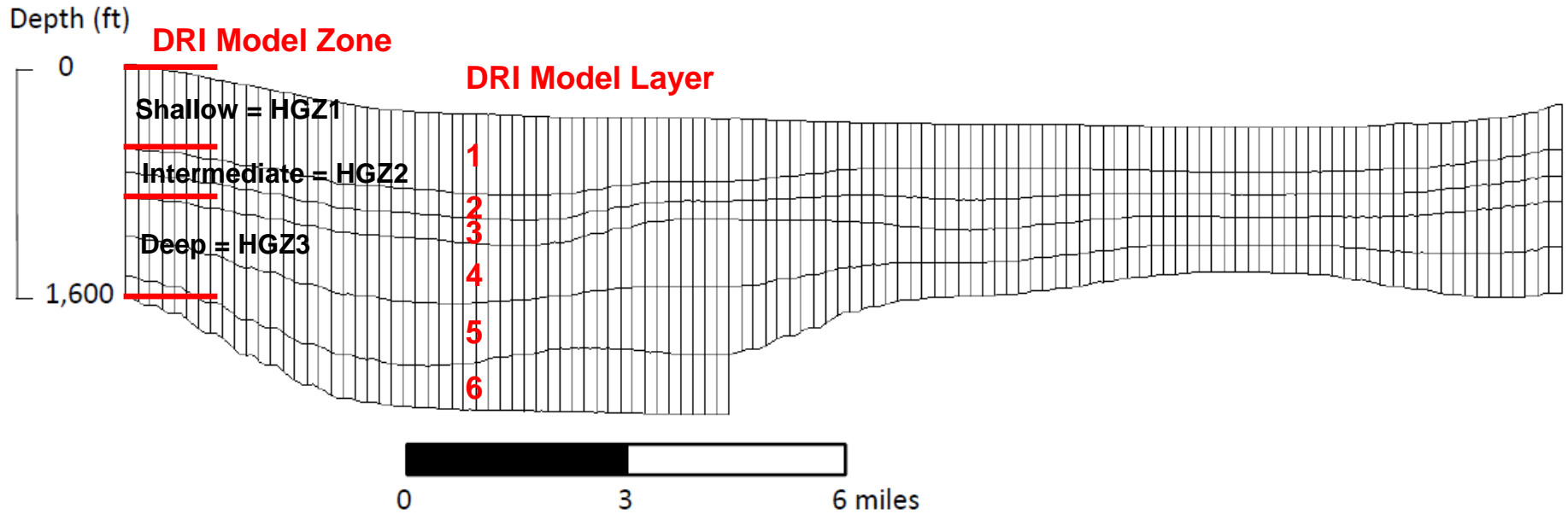
HYDROSTRATIGRAPHIC FEATURES

- 1 - FINES PLUG
- 2 - GRAVEL ZONE
- 3 - HIGH GRADIENT
- 4 - PLAYA
- LITTLE LAKE FAULT
- ACTIVE MODEL DOMAIN



Reference: Brown and Caldwell, 2009. Final Report: Indian Wells Valley Basin Groundwater Flow Model and Hydrogeologic Study. Prepared for Indian Wells Valley Water District, dated March 27.

Indian Wells Valley Technical Working Group	
Indian Wells Valley Brown & Caldwell CHM Geometry	
Date: 9/18/2023	Figure 10



DRI Model Zone	Hydrogeologic Zone	Model Layers
Shallow	HGZ1	1
Intermediate	HGZ2	2 and 3
Deep	HGZ3	4, 5, and 6

Reference: DRI, 2018. Indian Wells Valley Groundwater Model. Presentation to TAC. Slide 16 – Flow Model, dated October 4.

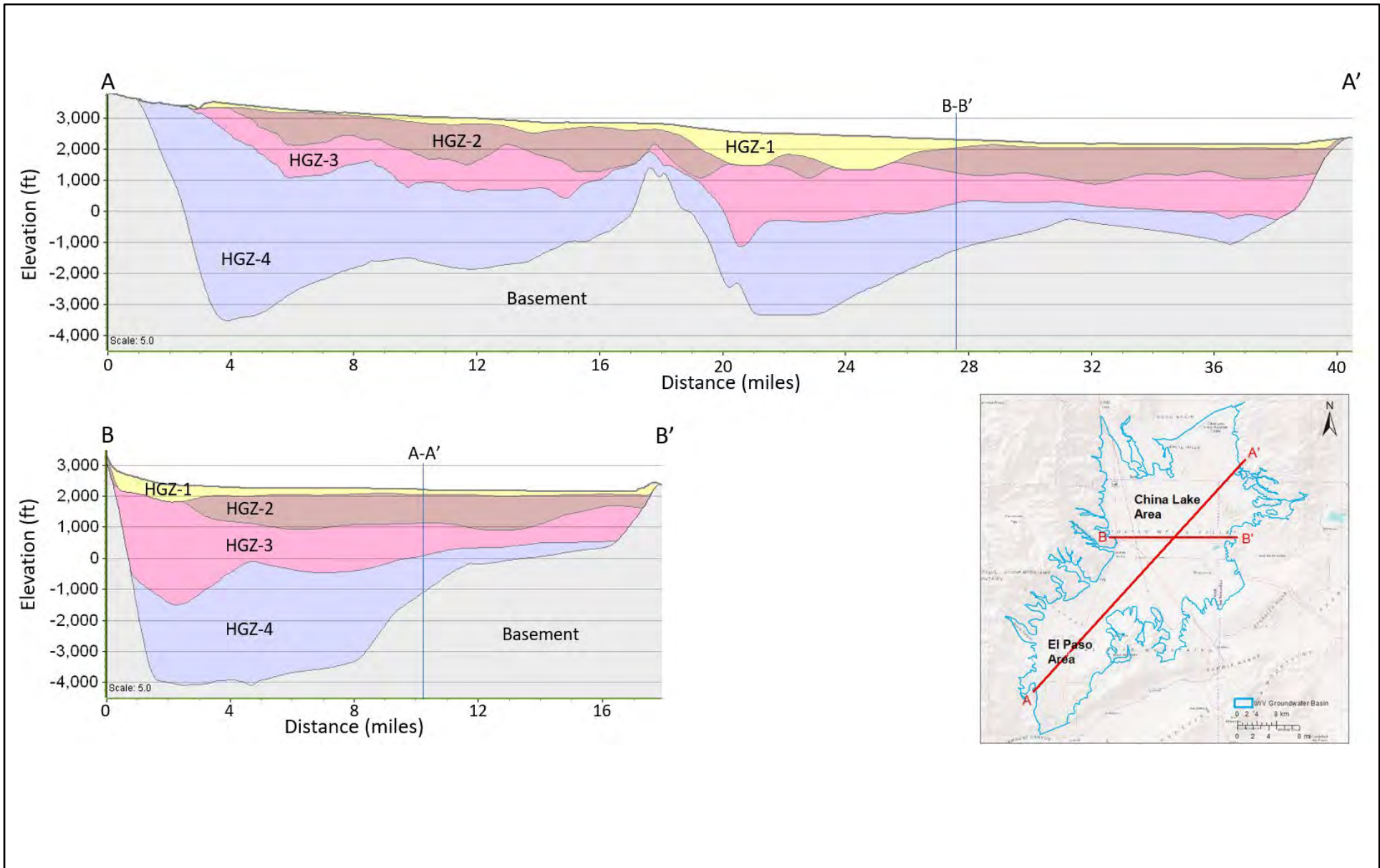
Modified to include explanatory labels.

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Indian Wells Valley
DRI Model Layers

Date: 9/18/2023

Figure 11



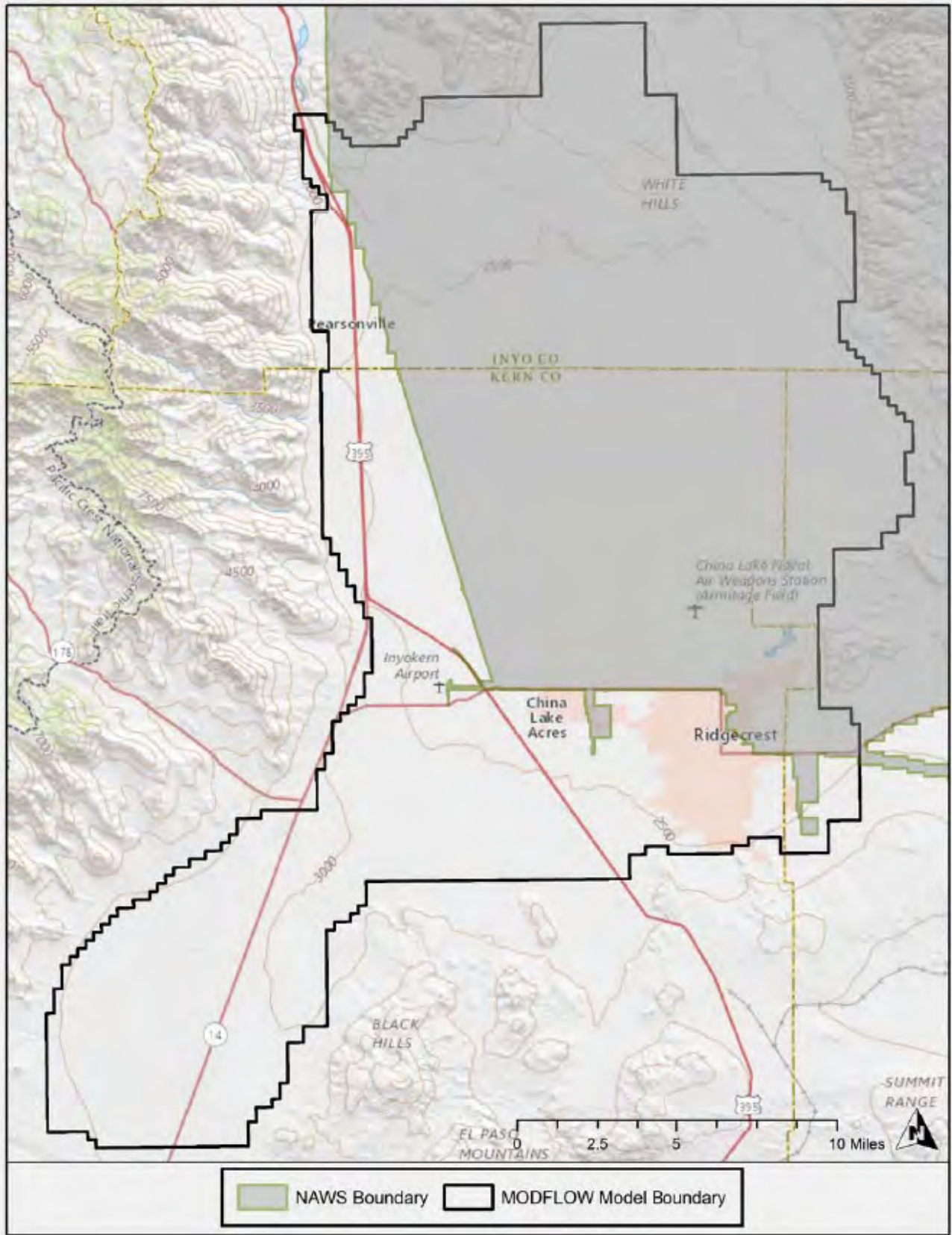
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Indian Wells Valley
Ramboll 4-Zone HCM

Date: 9/18/2023

Figure 12



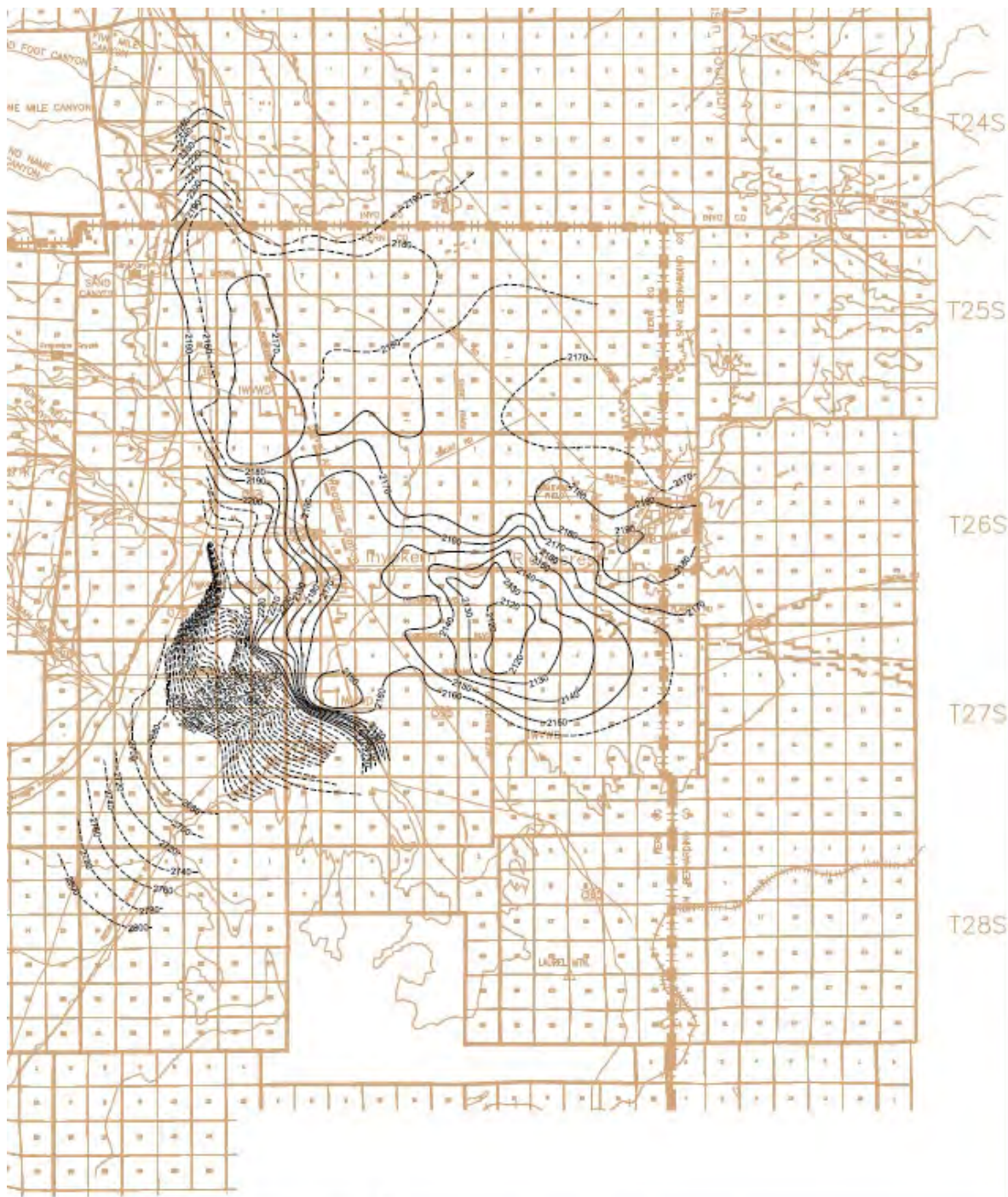
Reference: Stetson Engineers, 2017. Indian Wells Valley Model Review of DRI 2017 Model Update. Figure 1, dated December 7.



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Indian Wells Valley
DRI Model Boundary

Date: 9/18/2023

Figure 13



 STREAM RECORDING STATION (KOWA)
 CONTOUR INTERVAL IN NORTHERN PORTION OF MAP=10'
 CONTOUR INTERVAL IN SOUTHERN PORTION OF MAP=20'
 DASHED CONTOUR LINES INDICATE ESTIMATED VALUES
 GROUNDWATER BASIN BOUNDARY
 (CA Dept. of Water Resources, Bulletin 110)

Kern County Water Agency
 Kern County, California

**INDIAN WELLS VALLEY
 GROUNDWATER SURFACE ELEVATION
 SPRING 2015**

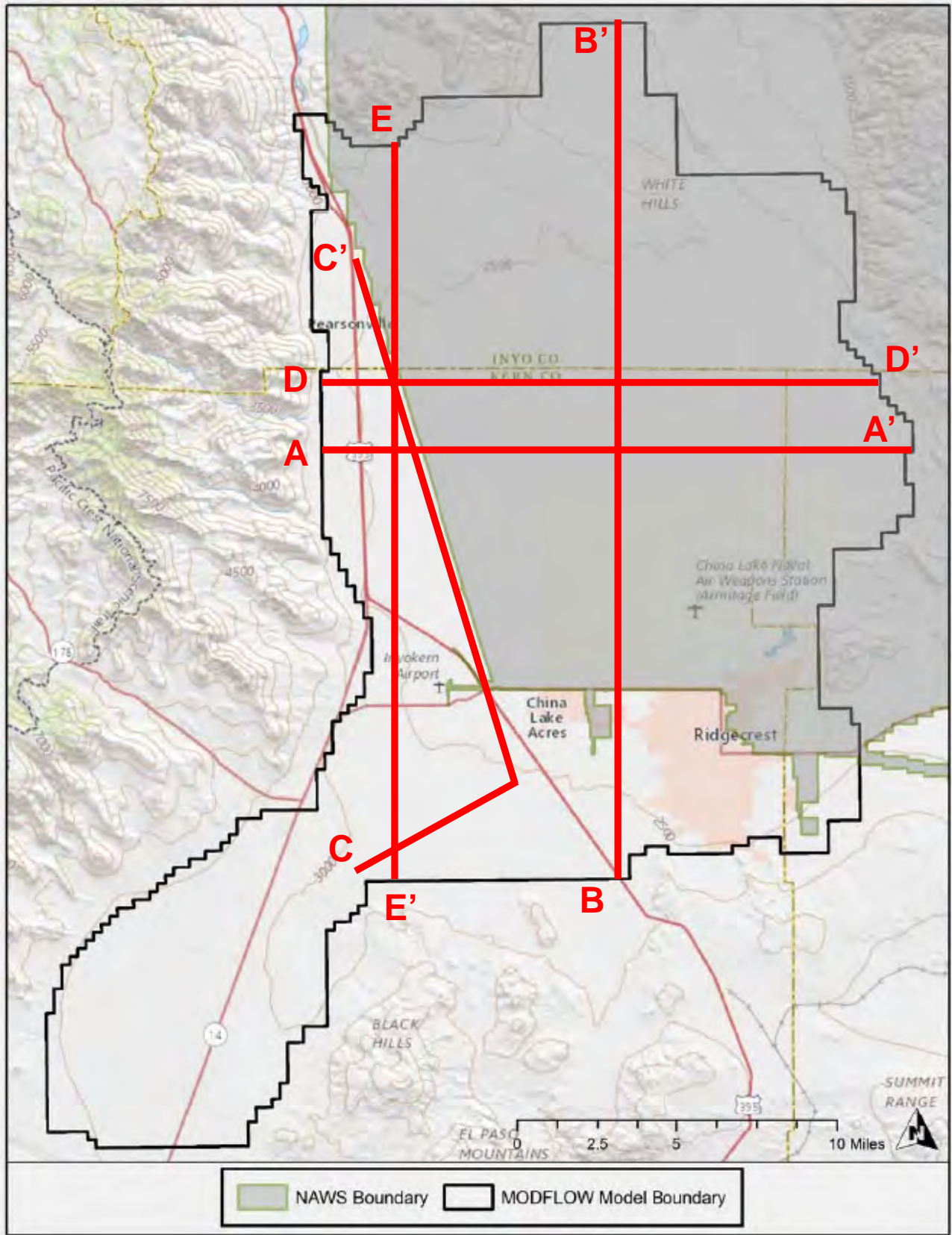
PLATE 1

M. Anderson PC #8898 (based on NIMA Survey Data) September 2015

Reference: Kern County Water Agency, 2015. Indian Wells Valley Groundwater Surface Elevation Spring 2015. Plate 1, dated September.

Indian Wells Valley
 Technical Working Group
 Indian Wells Valley
**Spring 2015 Groundwater
 Surface Elevation**

Date: 9/18/2023 Figure 14



Reference: Stetson Engineers, 2017. Indian Wells Valley Model Review of DRI 2017 Model Update. Figure 1, dated December 7.

Modified to show cross-section locations.

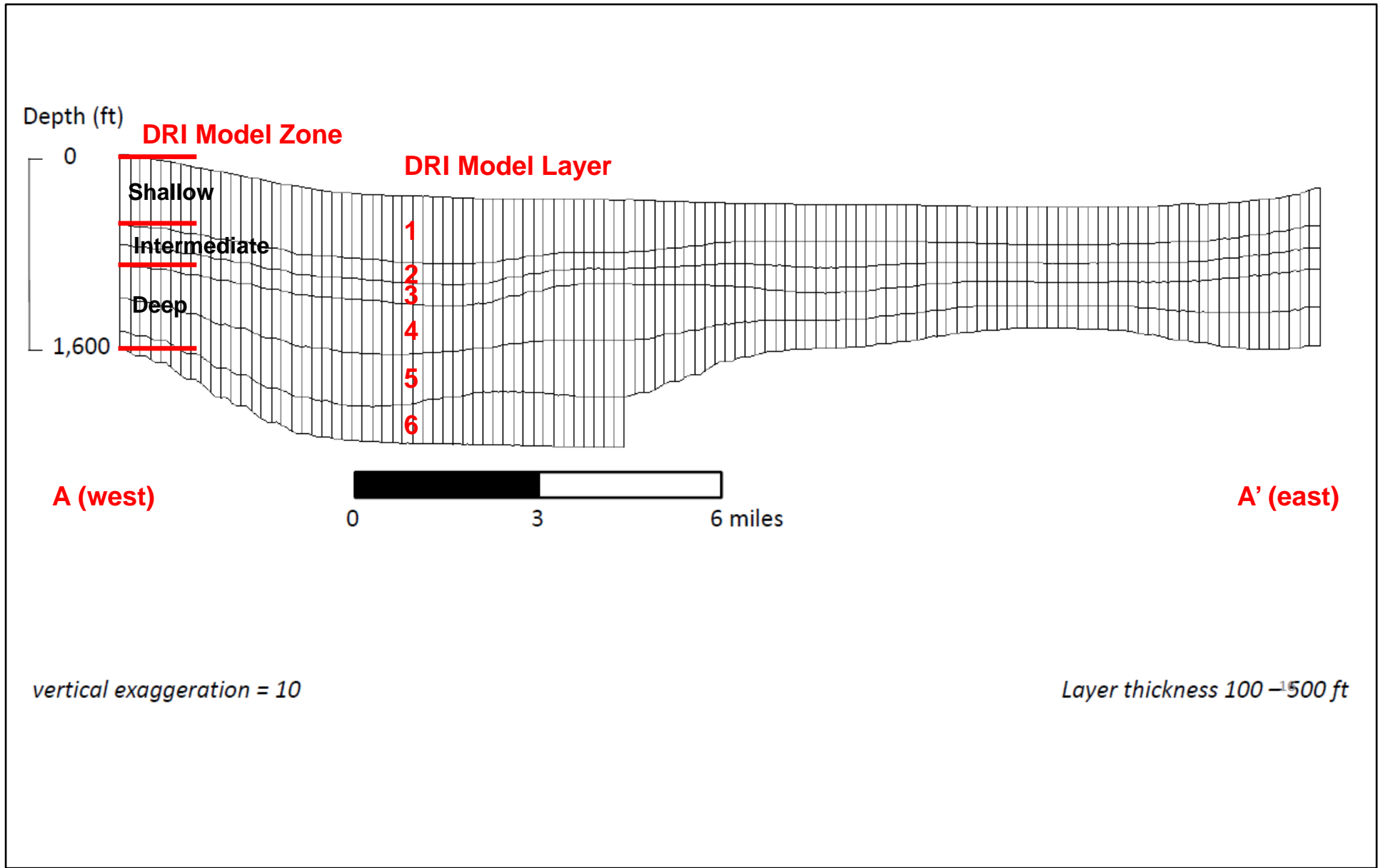
Indian Wells Valley
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Indian Wells Valley

Cross-Section Locations

Date: 9/18/2023

Figure 15



Reference: DRI, 2018. Indian Wells Valley Groundwater Model. Presentation to TAC. Slide 16 – Flow Model, dated October 4.

Modified to include explanatory labels.

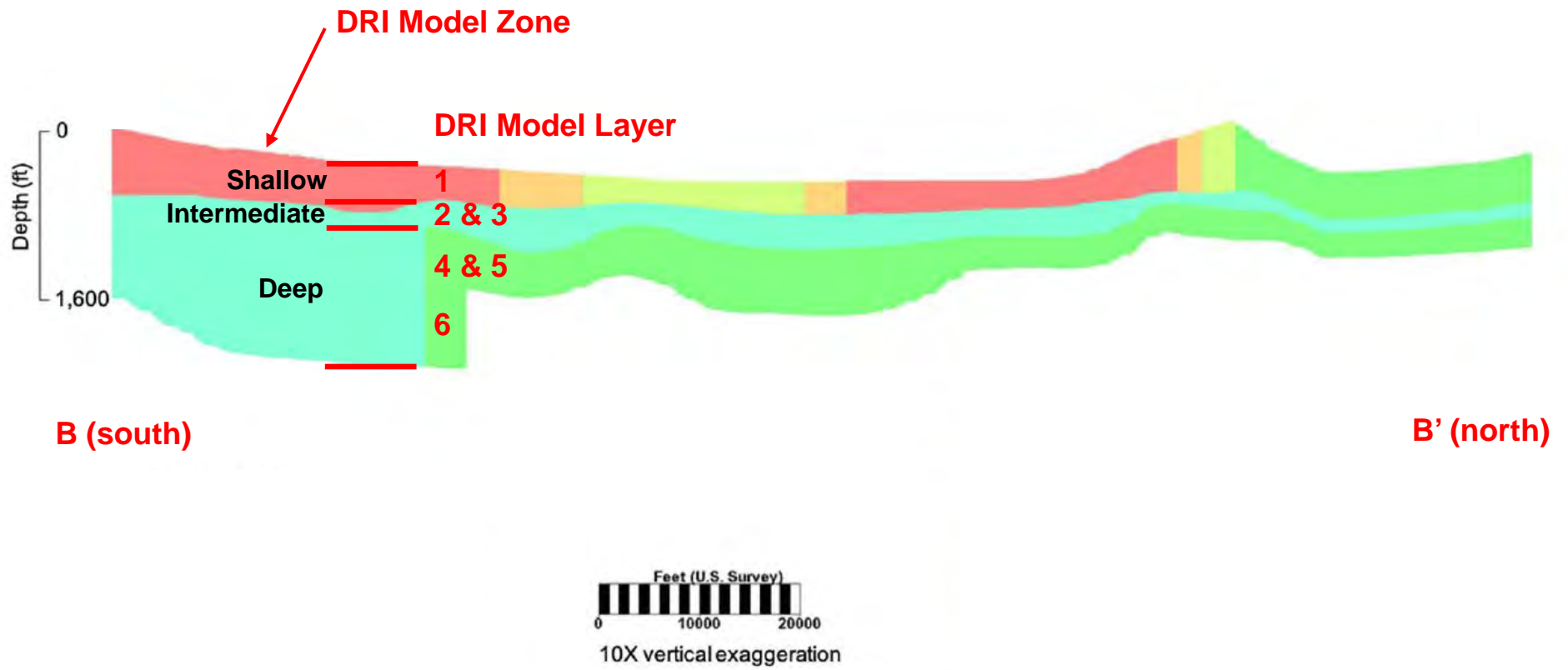
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Indian Wells Valley

DRI Model Cross-Section A-A'

Date: 9/18/2023

Figure 16



Note: The colors on the cross-section are the DRI-assumed hydraulic conductivities. This analysis only used the colors to define the shape of the boundaries between the various model layers.

Reference: Stetson Engineers, 2020. Groundwater Sustainability Plan for the Indian Wells Valley Groundwater Basin. Bulletin 118 Basin No. 6-054. Indian Wells Groundwater Authority. Appendix 3-H – Model Documentation. DRI Figure 15, dated January.

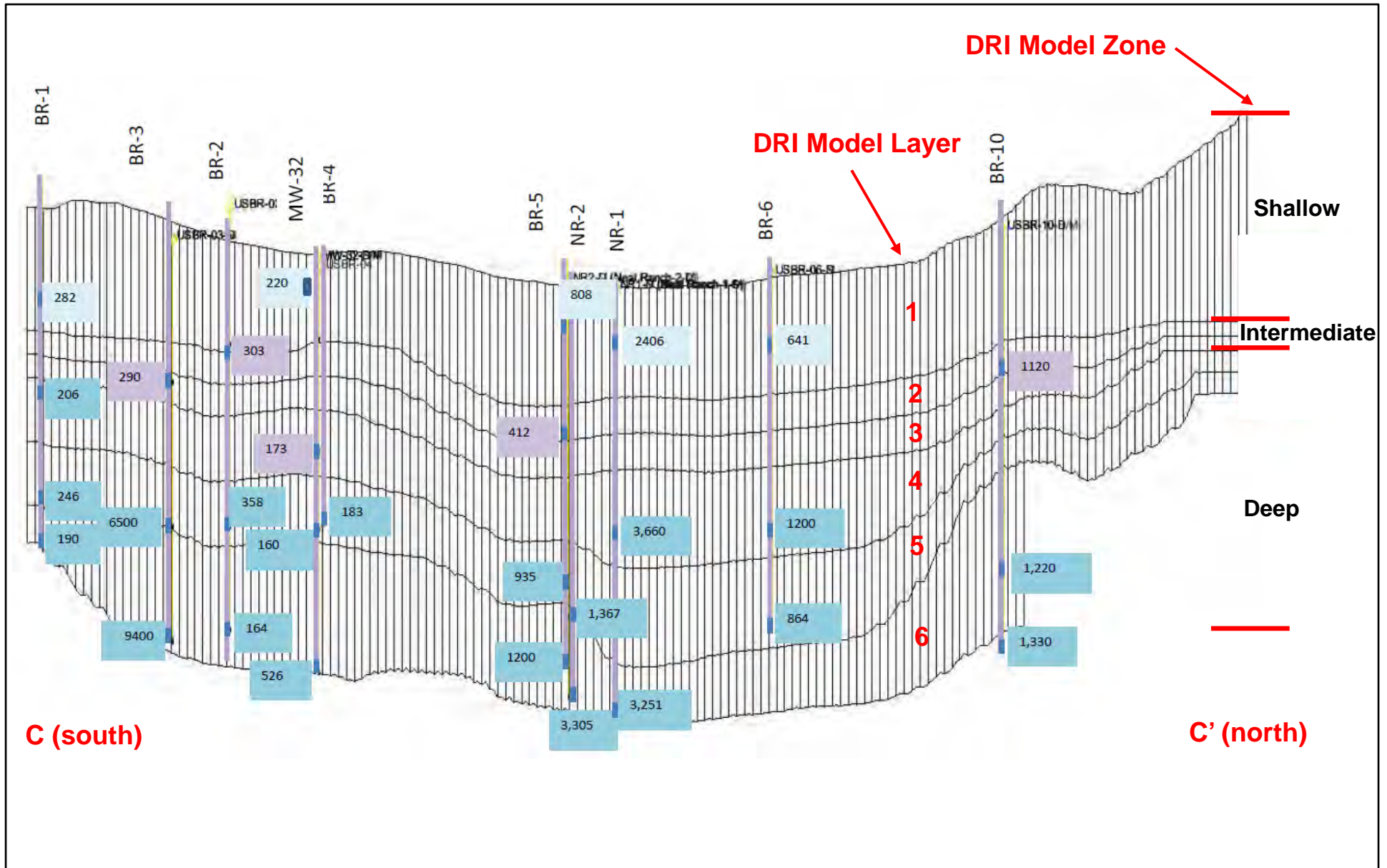
Modified to include explanatory labels.

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 Indian Wells Valley

DRI Model Cross-Section B-B'

Date: 9/18/2023

Figure 17



Reference: DRI, 2019. Indian Wells Valley Draft TDS Transport Model Baseline Pumping Conditions. Presentation to IWV TAC. Slide 7, dated February 7.

Modified to include explanatory labels.

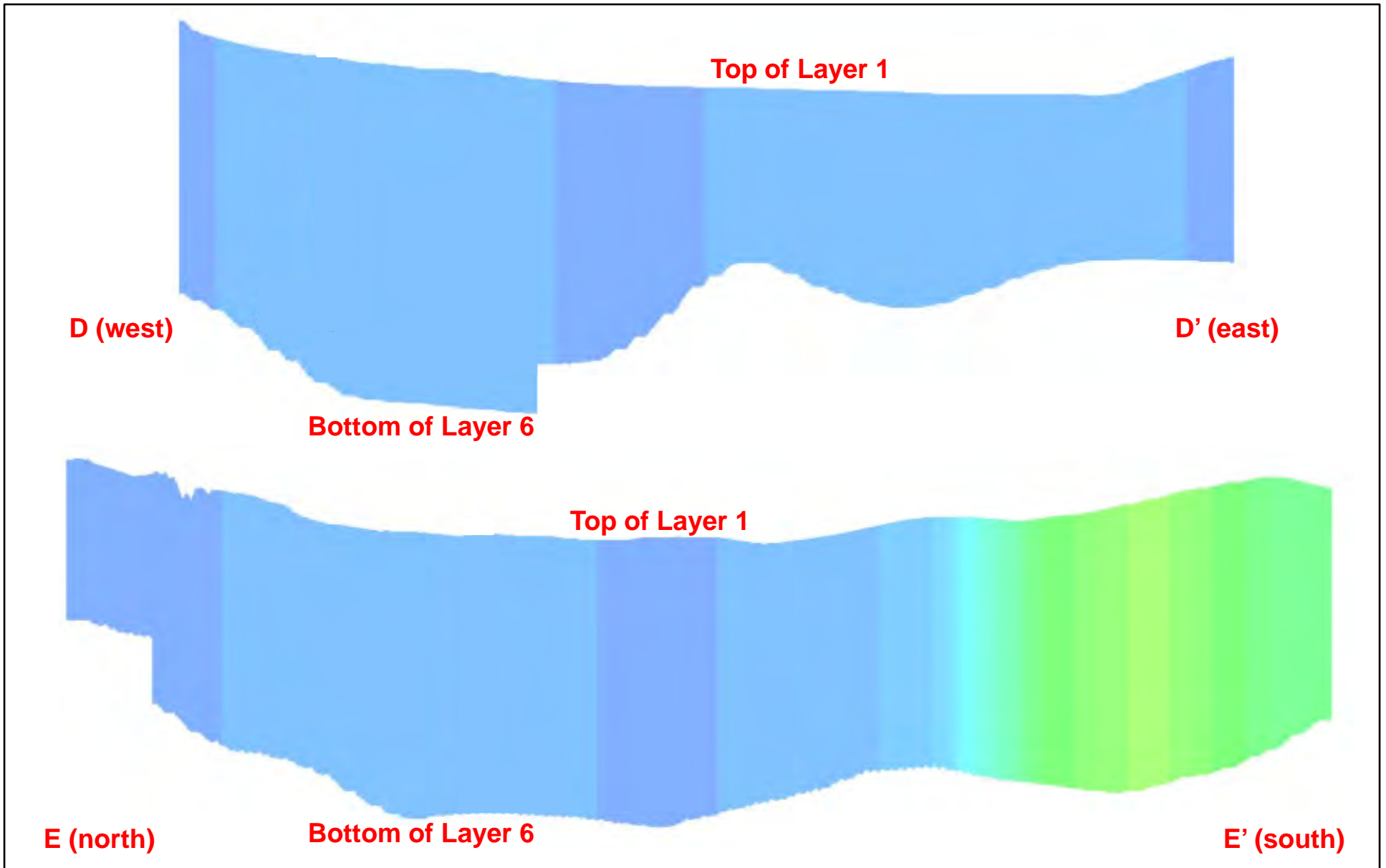
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Indian Wells Valley

DRI Model Cross-Section C-C'

Date: 9/18/2023

Figure 18



Reference: DRI, 2019. IWV Model Cross Sections – Hydraulic Conductivity and Specific Yield. Personal Communication. Mr. Christopher Garner. August 13.

Modified to include explanatory labels.

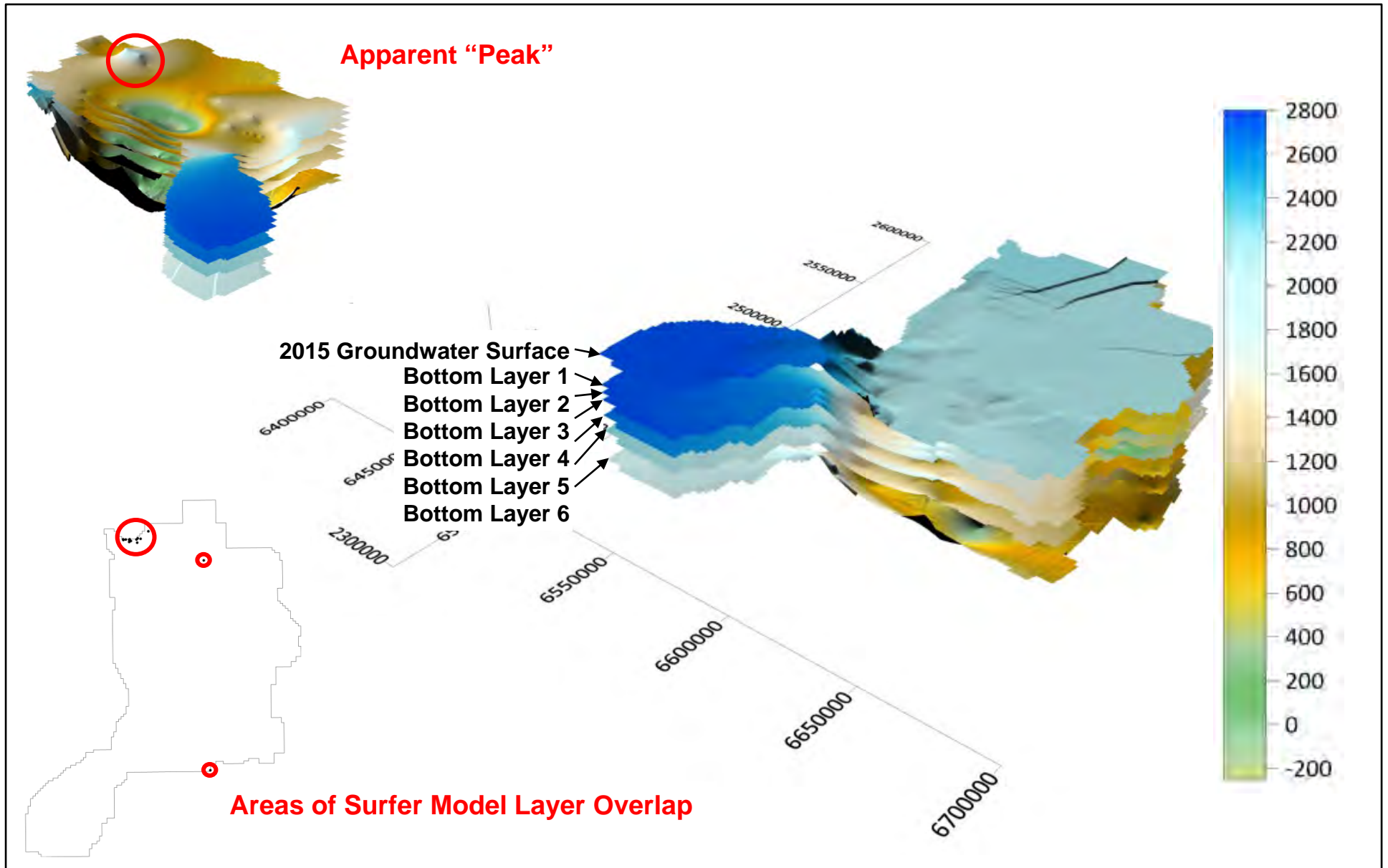
Note: The colors on the cross-sections are the DRI-assumed specific yield.

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 Indian Wells Valley

DRI Model
Cross-Sections D-D' & E-E'

Date: 9/18/2023

Figure 19



The large apparent “peak” in **Figure 20** results from the increase in elevations of that layer in the cross-sections as shown on **Figure 17** for cross-section B-B’, and **Figure 18** for cross-section C-C’. The vertical to horizontal exaggeration for **Figure 20** is approximately 23 to 1, making the “peak” appear much larger than it is in reality.

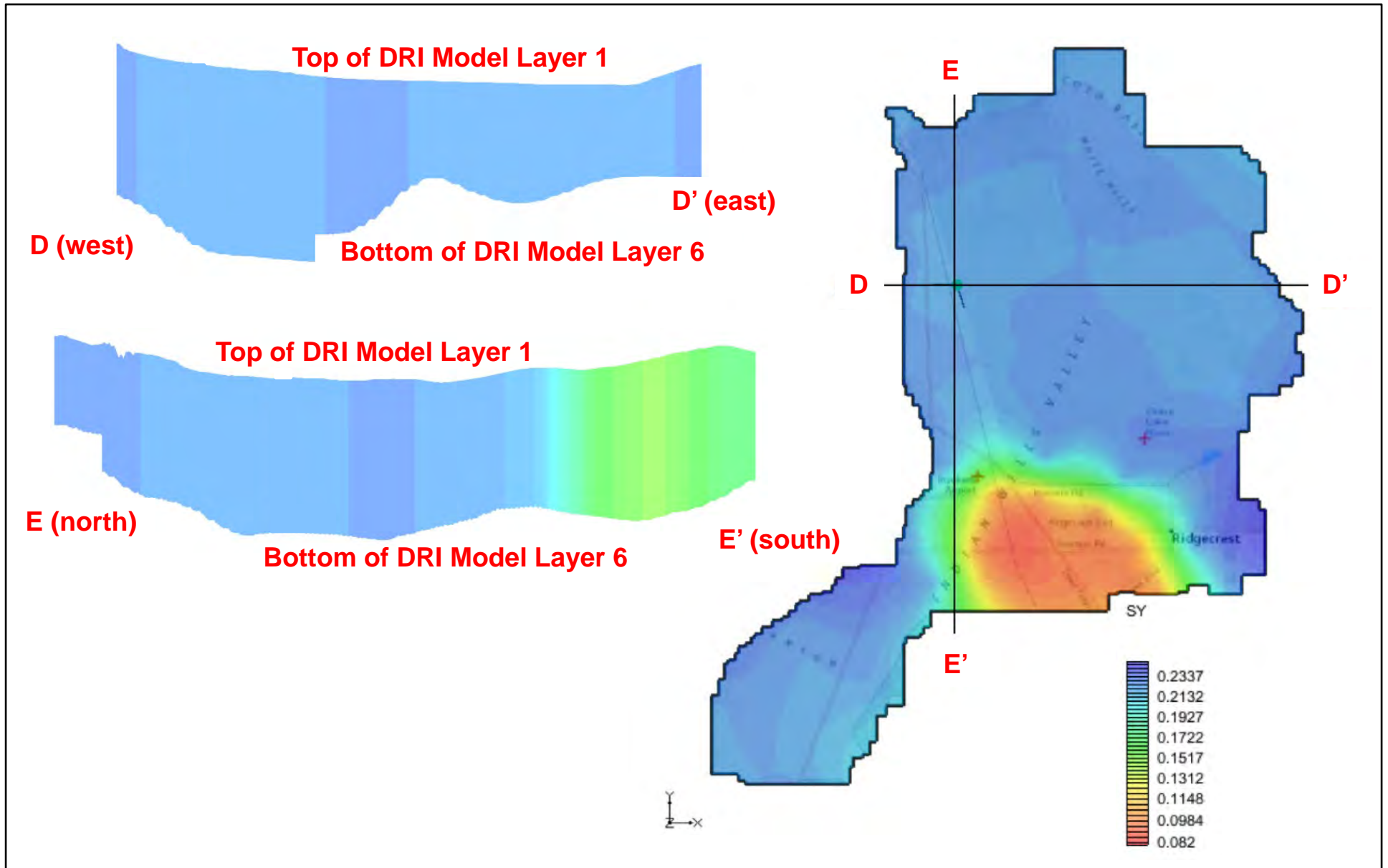
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Indian Wells Valley

Overlap of Surfer Model Layers

Date: 9/18/2023

Figure 20



Reference: DRI, 2019. IWV Model Cross Sections – Hydraulic Conductivity and Specific Yield. Personal Communication. Mr. Christopher Garner. August 13.

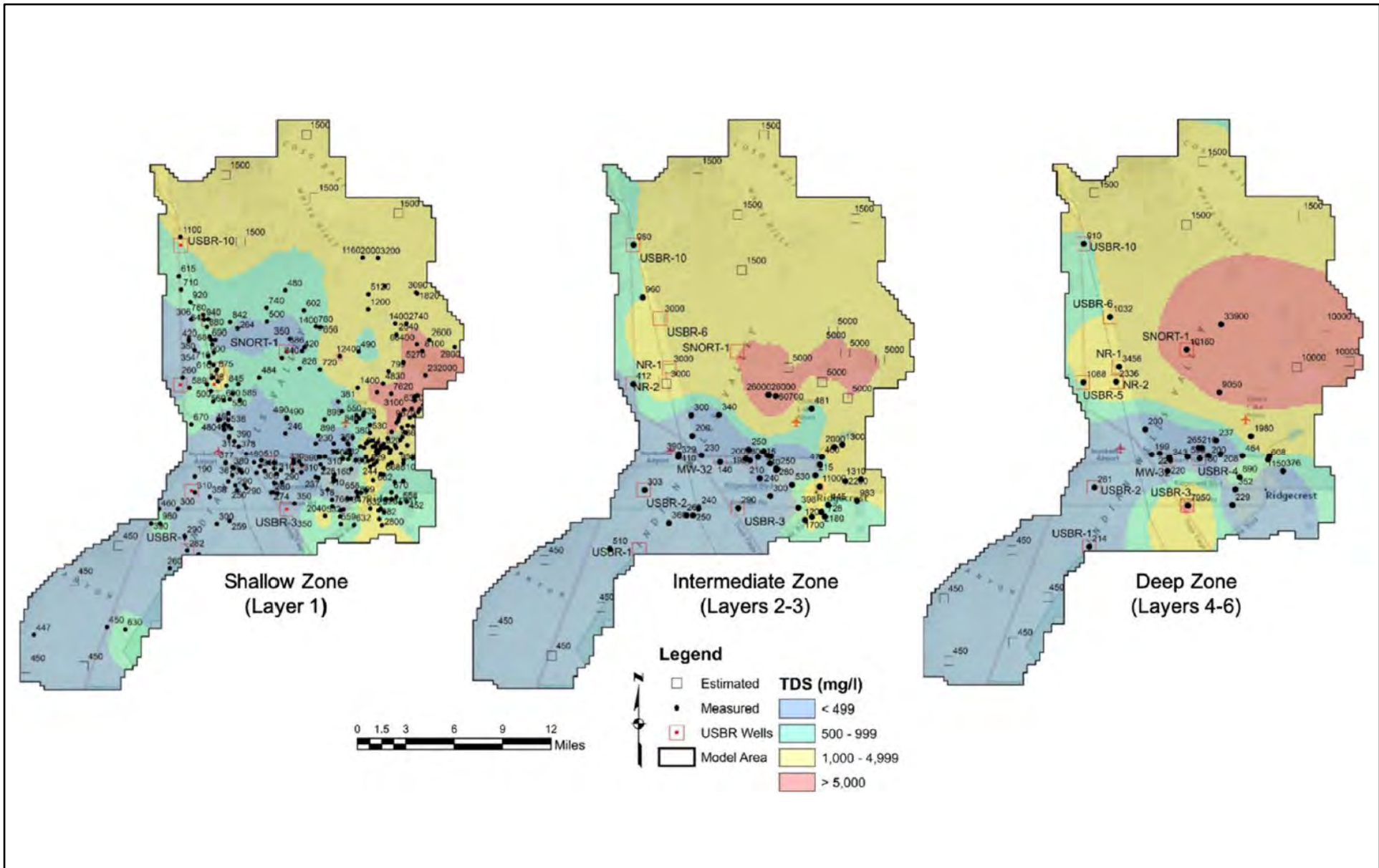
Modified to include explanatory labels.

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DRI Model
Specific Yield (Sy) Distribution

Date 9/18/2023

Figure 21



Reference: Stetson Engineers, 2020. Groundwater Sustainability Plan for the Indian Wells Valley Groundwater Basin. Bulletin 118 Basin No. 6-054. Indian Wells Groundwater Authority. Appendix 3-H – Model Documentation. DRI Figure 40, dated January.

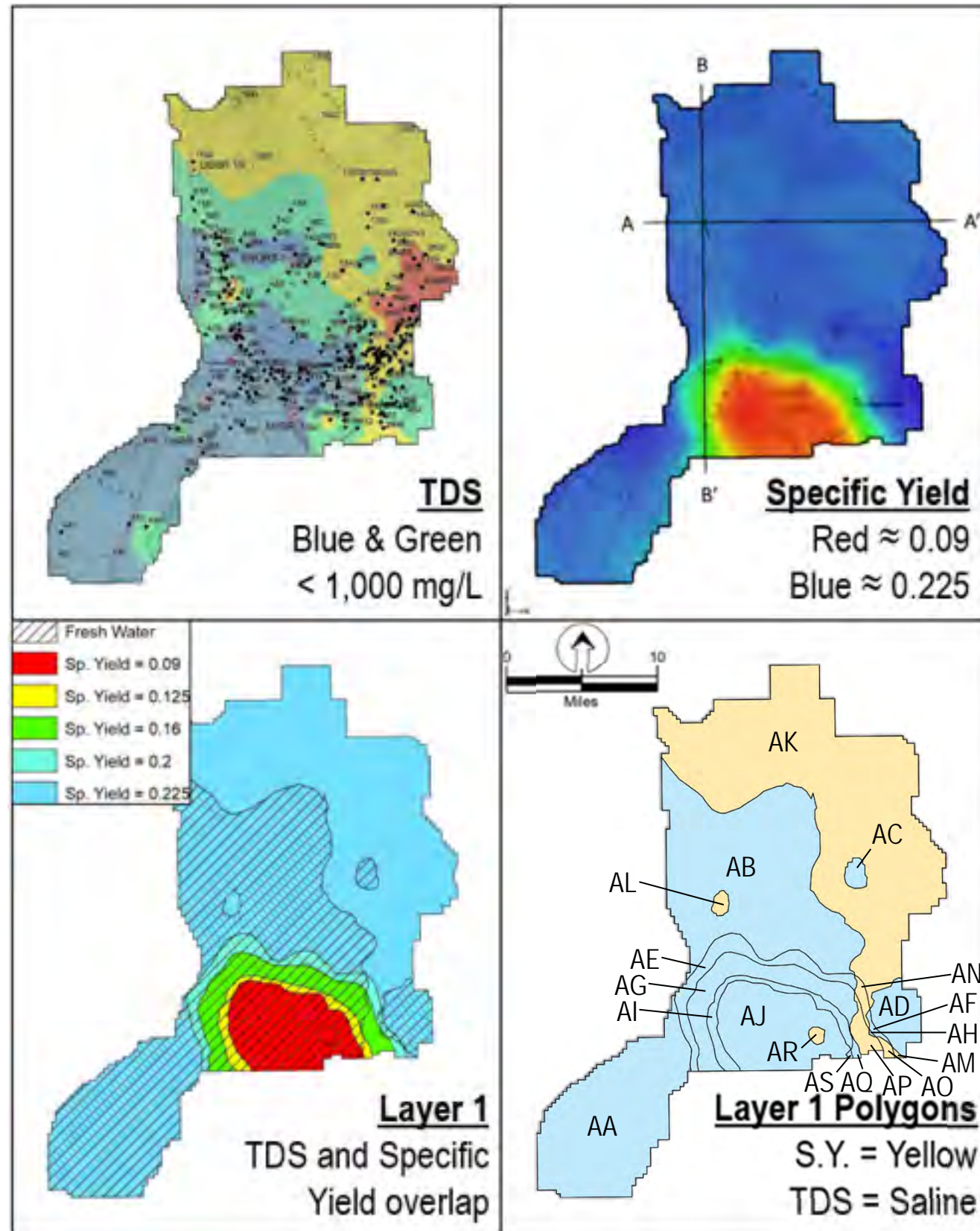
Modified to include explanatory labels.

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**DRI Model Distribution of
 Total Dissolved Solids (TDS)**

Date: 9/18/2023

Figure 22



Layer 1 Volumes (Acre-Feet)

Fresh Water

AA = 4,865,950
 AB = 4,437,468
 AC = 92,625
 AD = 212,728
 AE = 465,102
 AF = 8,103
 AG = 447,124
 AH = 683
 AI = 125,721
 AJ = 313,311

Brackish / Saline Water

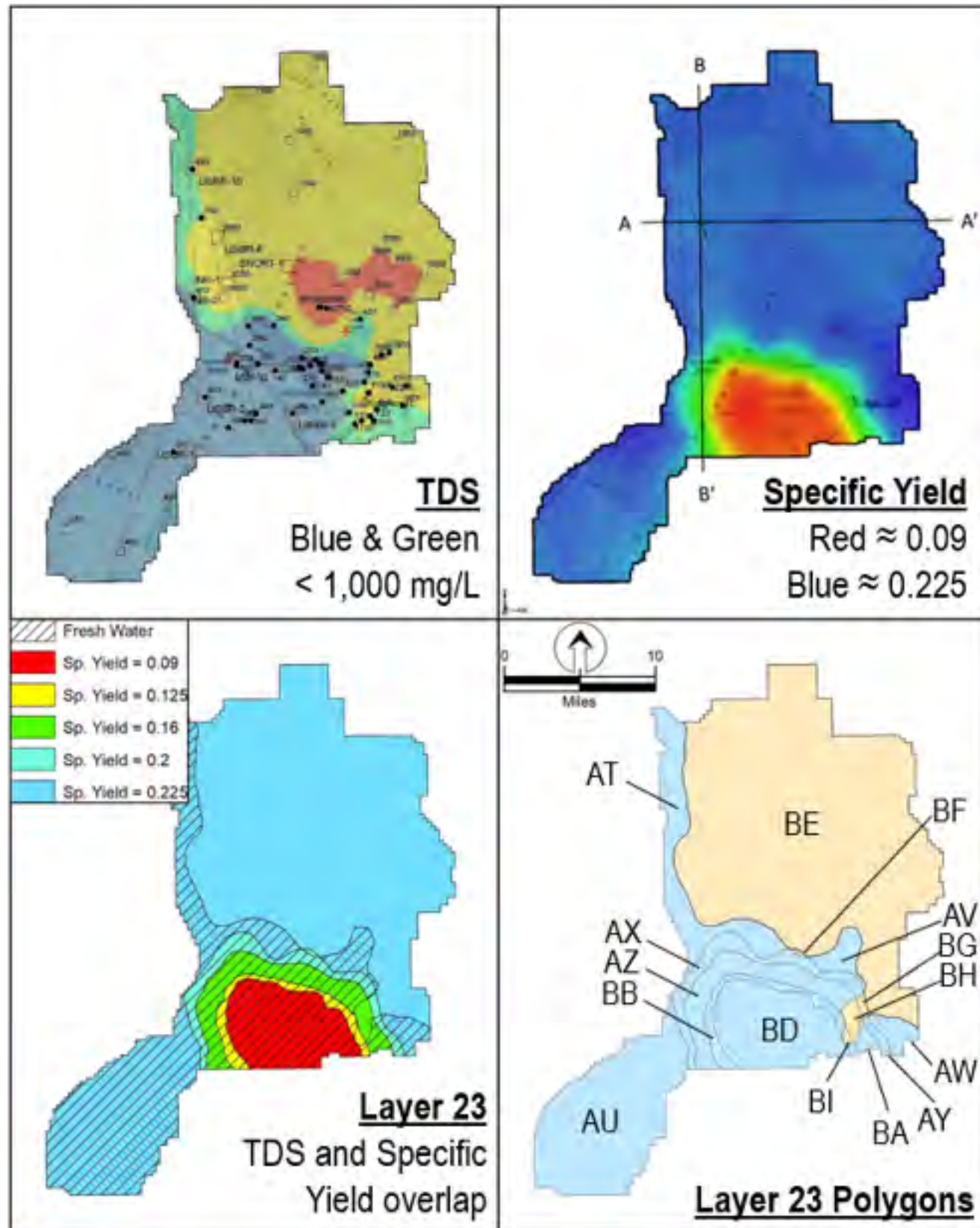
AK = 5,664,417
 AL = 80,352
 AM = 2,620
 AN = 24,825
 AO = 7,616
 AP = 24,633
 AQ = 2,115
 AR = 2,521
 AS = 339

Fresh Water Total

10,968,815

Brackish / Saline Water Total

5,809,437



Layer 2 Volumes (Acre-Feet)

Fresh Water

AT = 683,049
 AU = 1,226,127
 AV = 151,659
 AW = 97,926
 AX = 274,168
 AY = 26,897
 AZ = 308,253
 BA = 26,416
 BB = 89,236
 BD = 287,588

Brackish / Saline Water

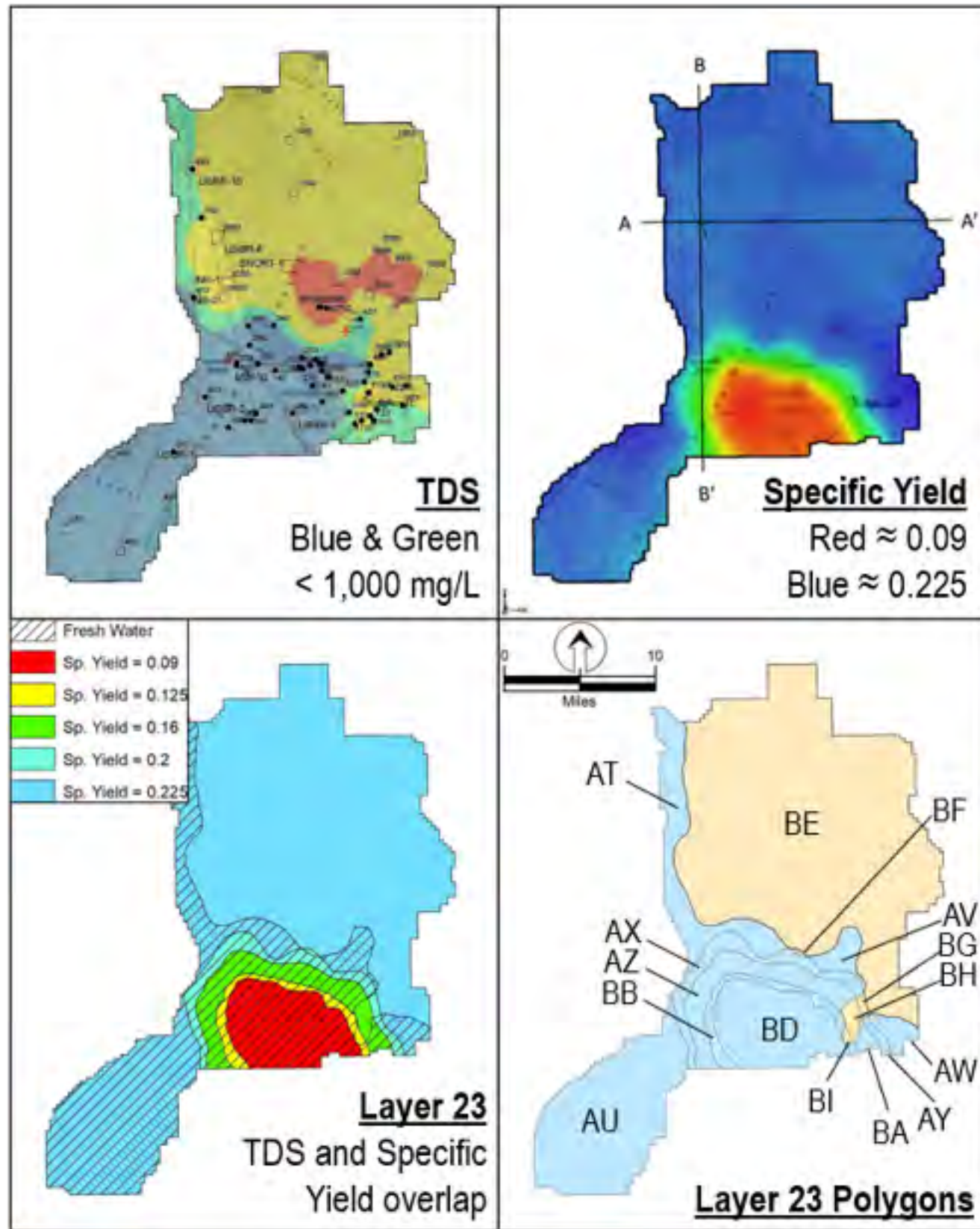
BE = 5,027,308
 BF = 4,982
 BG = 14,626
 BH = 30,383
 BI = 6,068

Fresh Water Total

3,171,319

Brackish / Saline Water Total

5,083,366



Layer 3 Volumes (Acre-Feet)

Fresh Water

AT = 678,865
 AU = 1,215,841
 AV = 152,112
 AW = 99,246
 AX = 273,675
 AY = 27,302
 AZ = 306,808
 BA = 26,844
 BB = 88,840
 BD = 286,178

Fresh Water Total

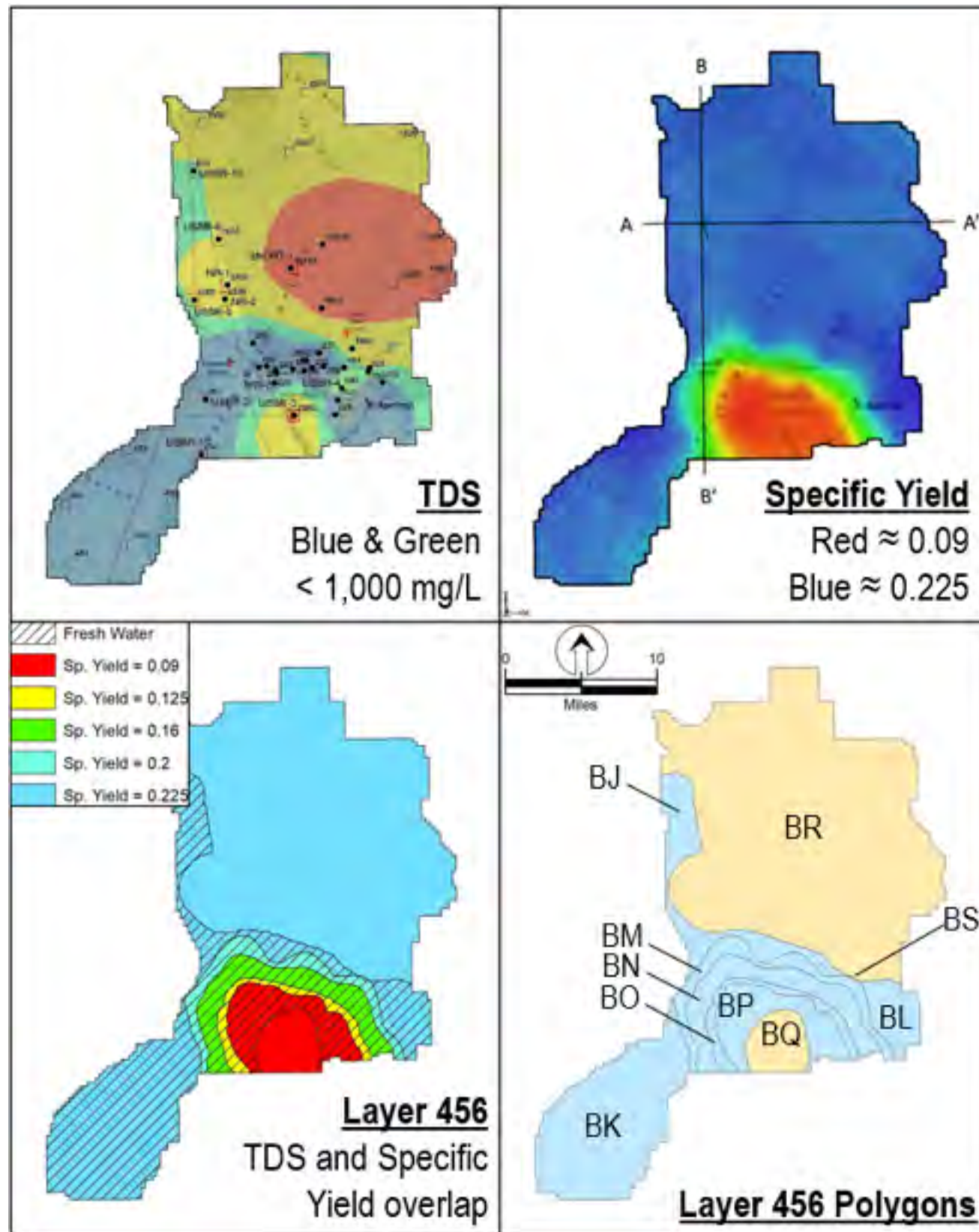
3,155,711

Brackish / Saline Water

BE = 5,026,083
 BF = 4,981
 BG = 14,773
 BH = 30,743
 BI = 6,163

Brackish / Saline Water Total

5,082,742



Layer 4 Volumes (Acre-Feet)

Fresh Water

BJ = 993,158
 BK = 2,681,842
 BL = 586,544
 BM = 582,818
 BN = 782,364
 BO = 230,331
 BP = 431,998

Brackish / Saline Water

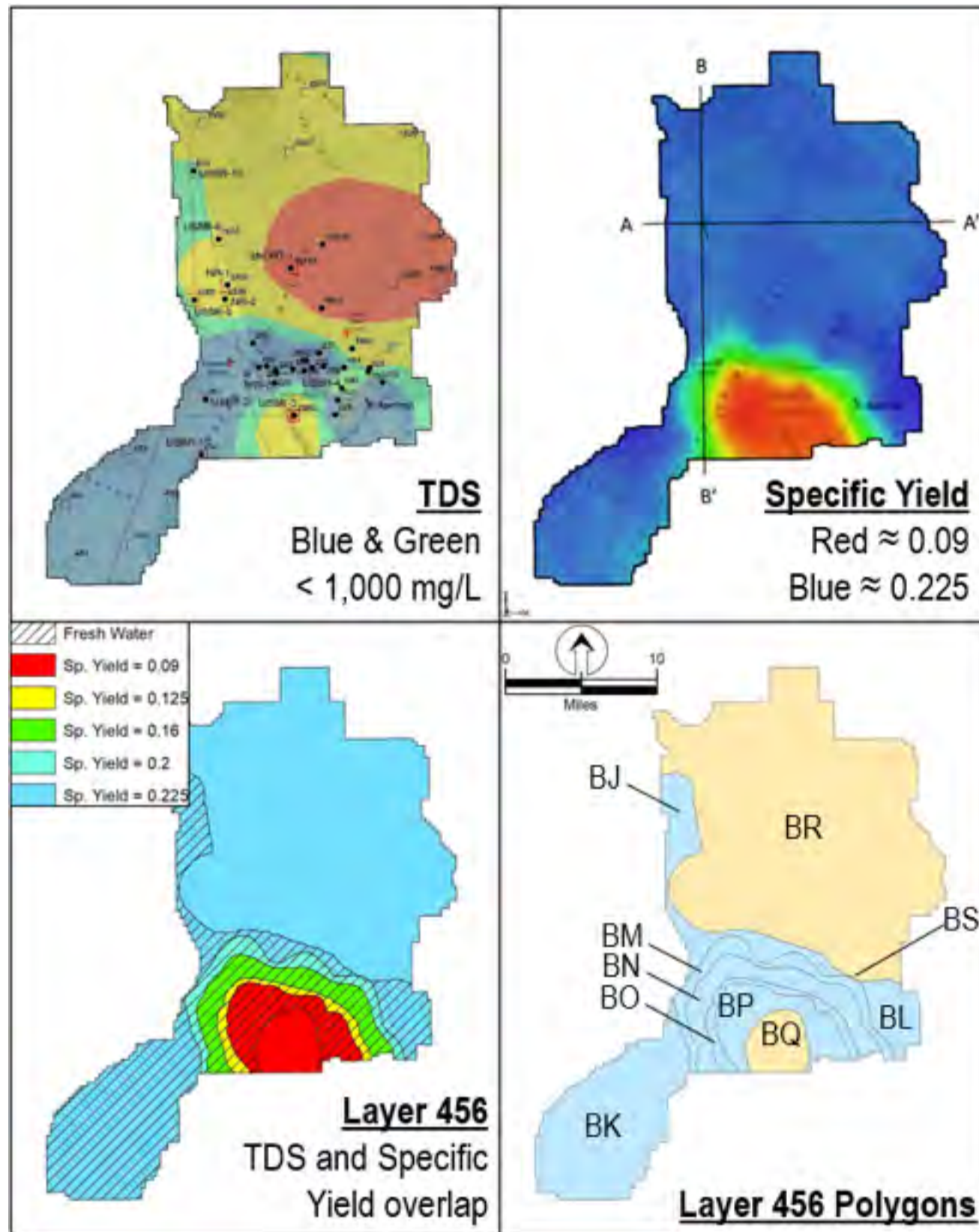
BQ = 260,235
 BR = 7,520,117
 BS = 3,588

Fresh Water Total

6,289,055

Brackish / Saline Water Total

7,783,940



Layer 5 Volumes (Acre-Feet)

Fresh Water

BJ = 995,203
 BK = 2,709,589
 BL = 587,228
 BM = 581,564
 BN = 781,455
 BO = 230,276
 BP = 432,361

Brackish / Saline Water

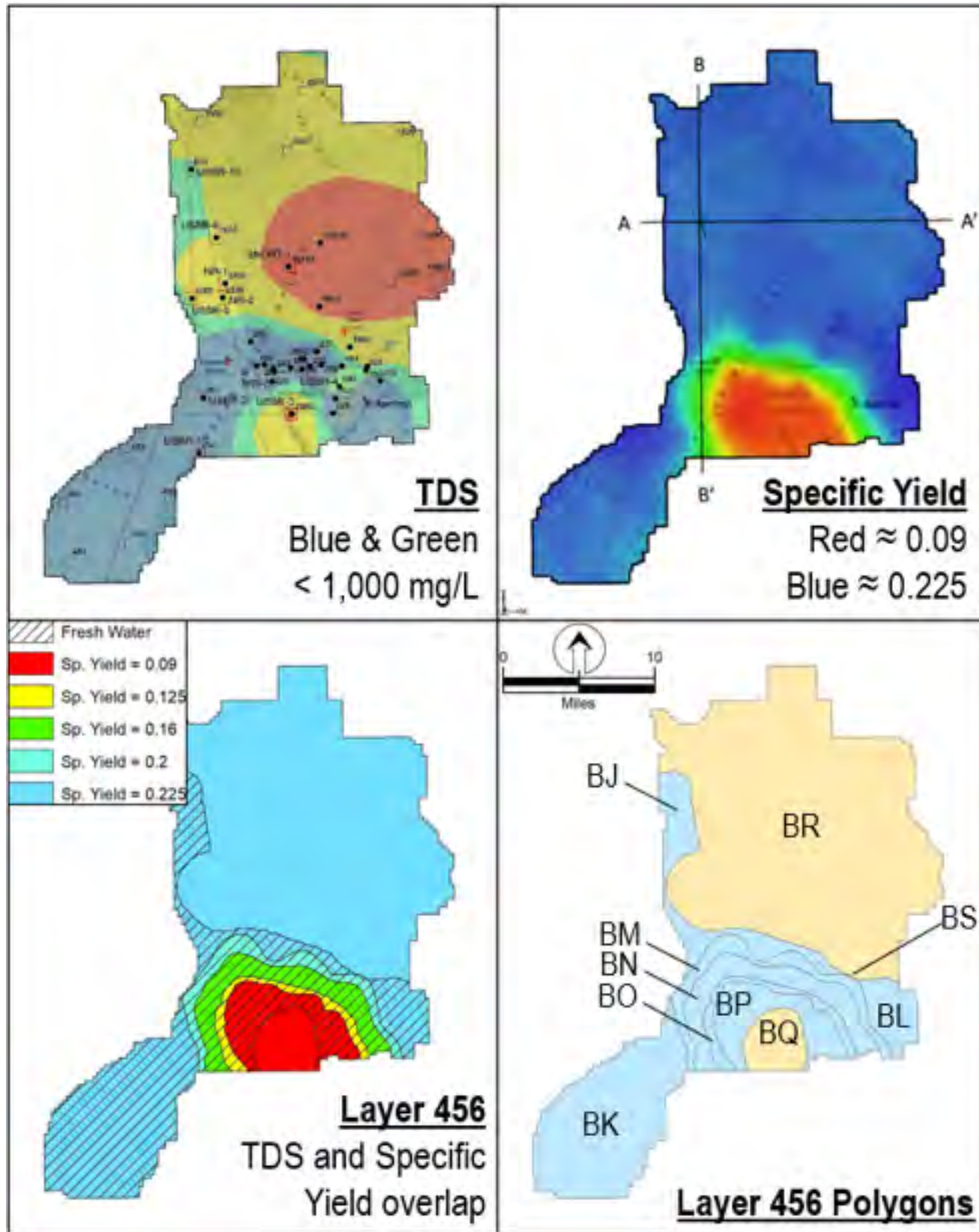
BQ = 259,625
 BR = 7,455,428
 BS = 3,593

Fresh Water Total

6,317,676

Brackish / Saline Water Total

7,718,646



Layer 6 Volumes (Acre-Feet)

Fresh Water

BJ = 2,132,830
 BK = 4,252,290
 BL = 1,197,746
 BM = 1,373,327
 BN = 1,775,860
 BO = 460,088
 BP = 872,571

Brackish / Saline Water

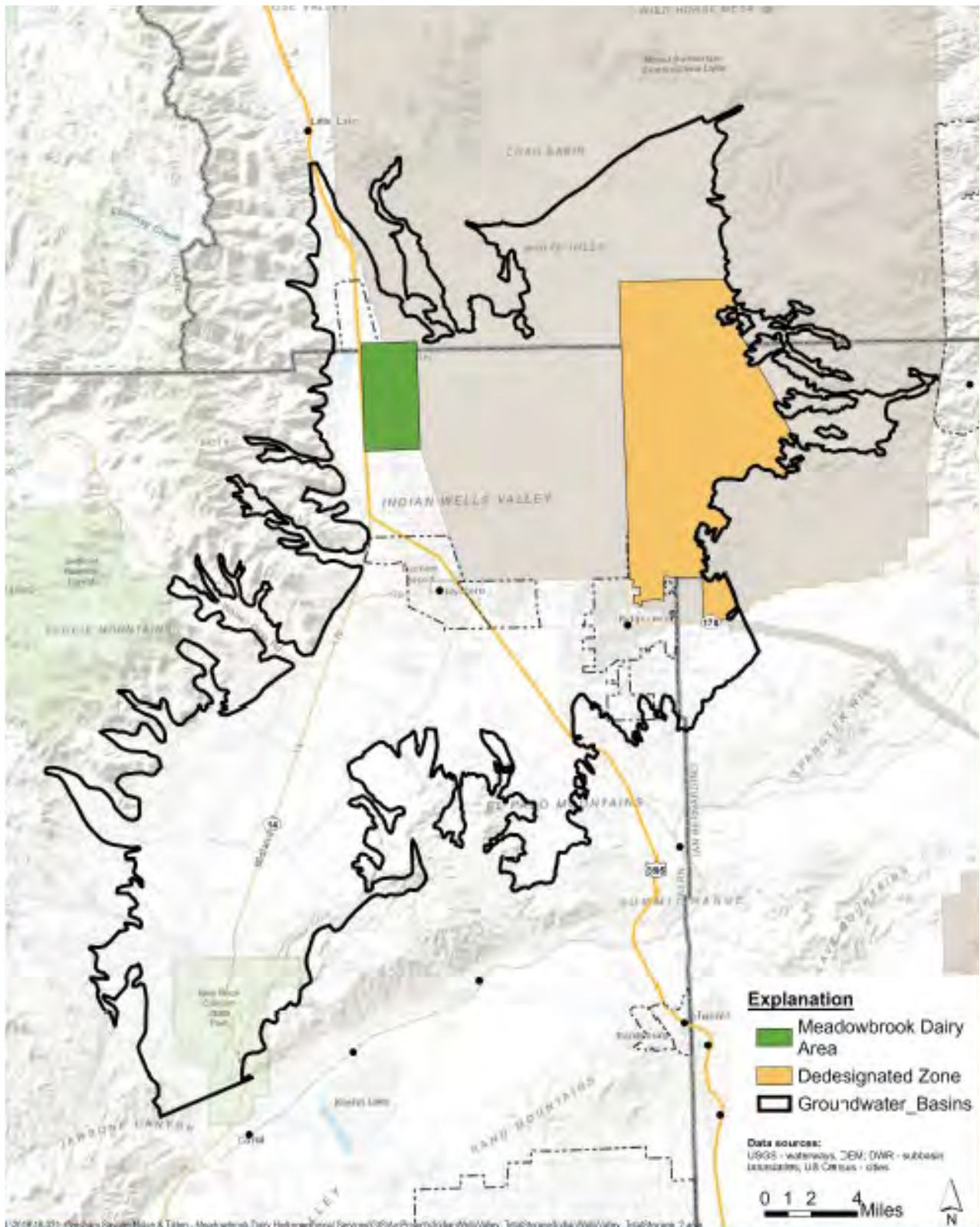
BQ = 458,818
 BR = 19,513,548
 BS = 9,039

Fresh Water Total

12,064,712

Brackish / Saline Water Total

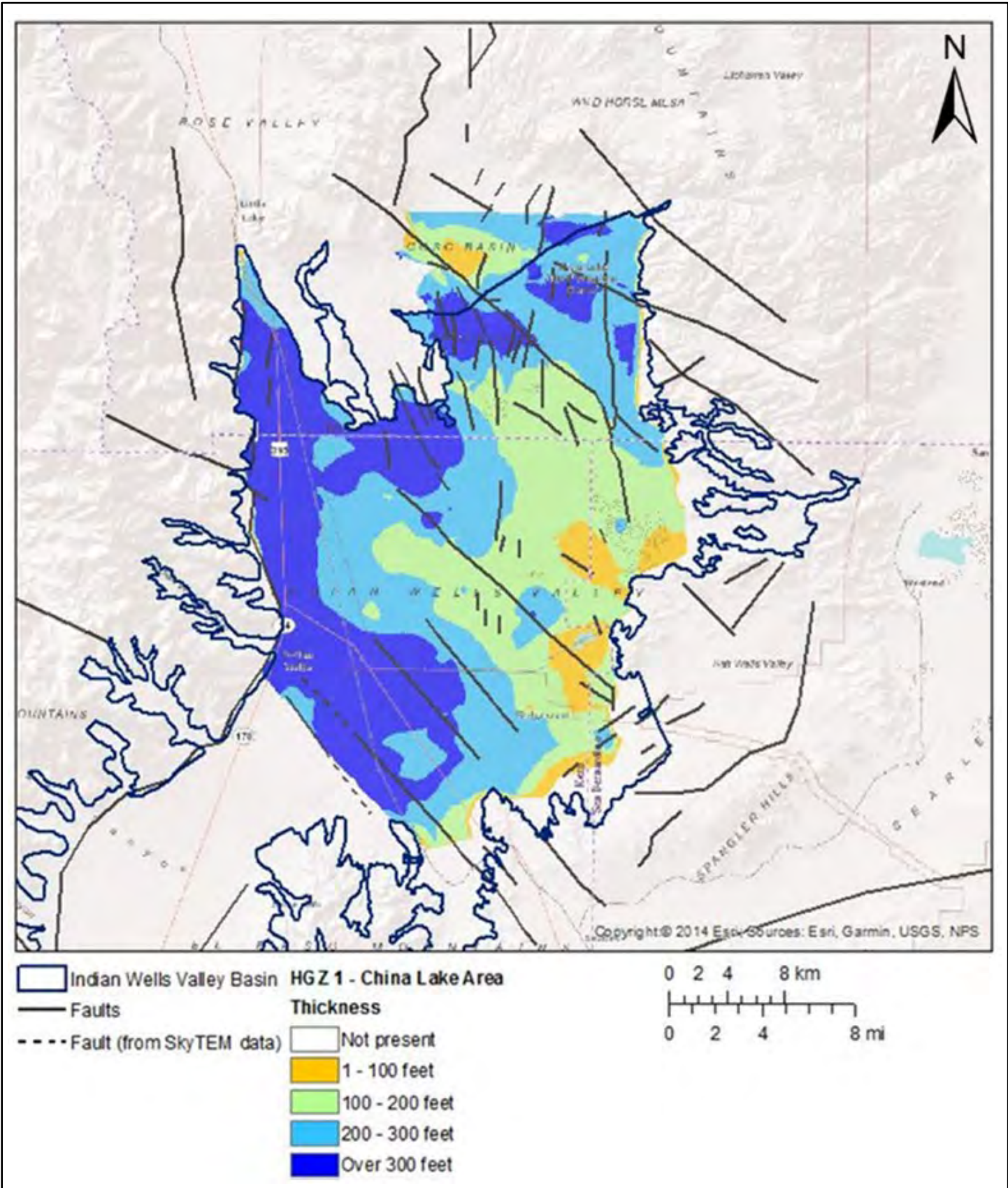
19,981,405



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Indian Wells Valley
**Groundwater Storage
Calculation Areas**

Date: 9/18/2023

Figure 29



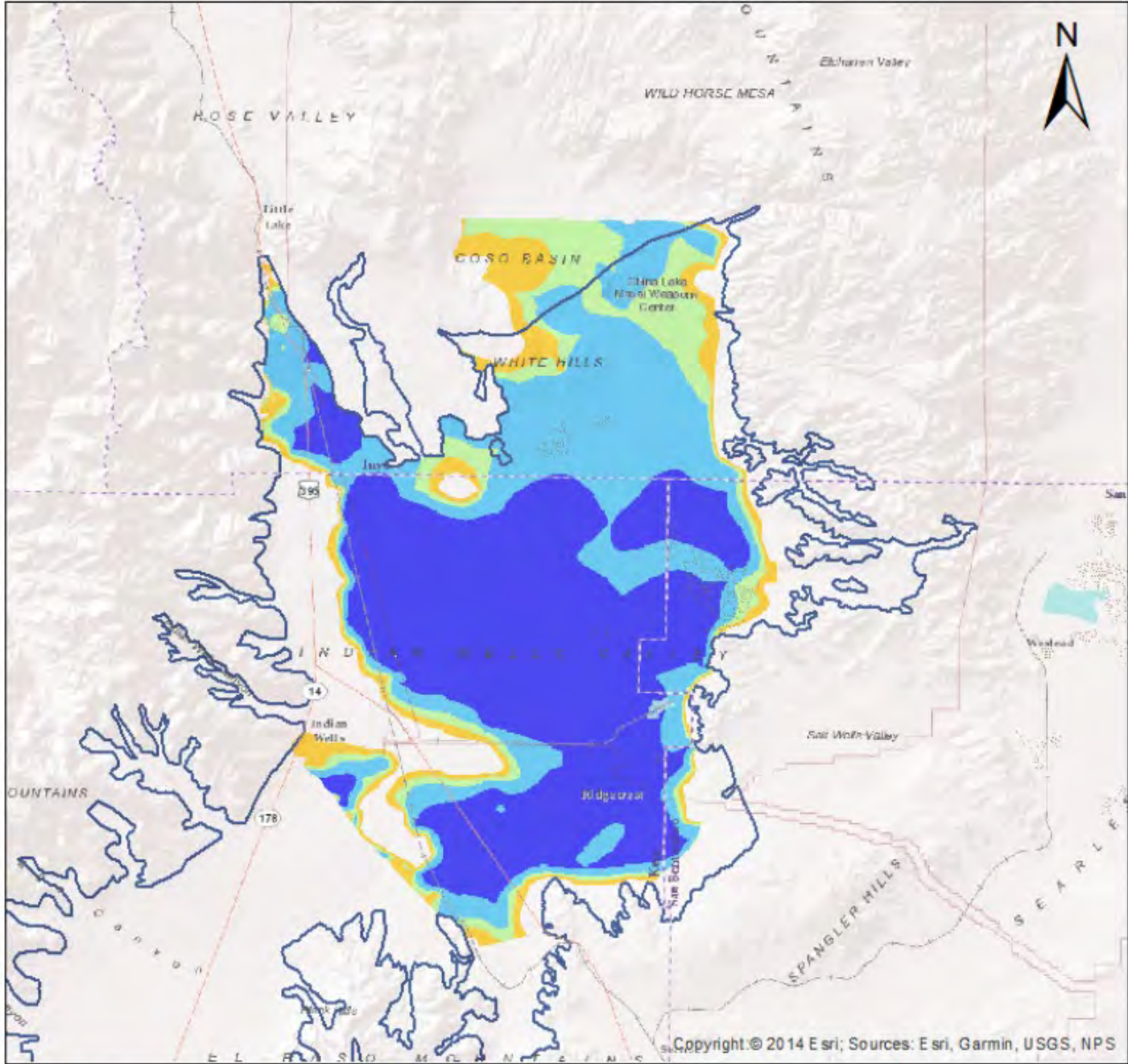
Reference: Ramboll, 2019. Hydrogeologic Conceptual Framework – Indian Wells Valley. Figure 6.5 – Thickness of the HGZ 1 in the China Lake area of the Indian Wells Valley Groundwater Basin, dated June.

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Indian Wells Valley
HGZ 1 Extent And Thickness

Date: 9/18/2023

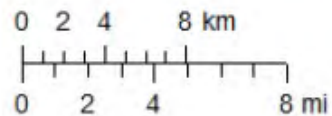
Figure 30



Indian Wells Valley Basin **HGZ2 - China Lake Area**

Thickness

- Not present
- 1 - 200 feet
- 200 - 400 feet
- 400 - 800 feet
- Over 800 feet



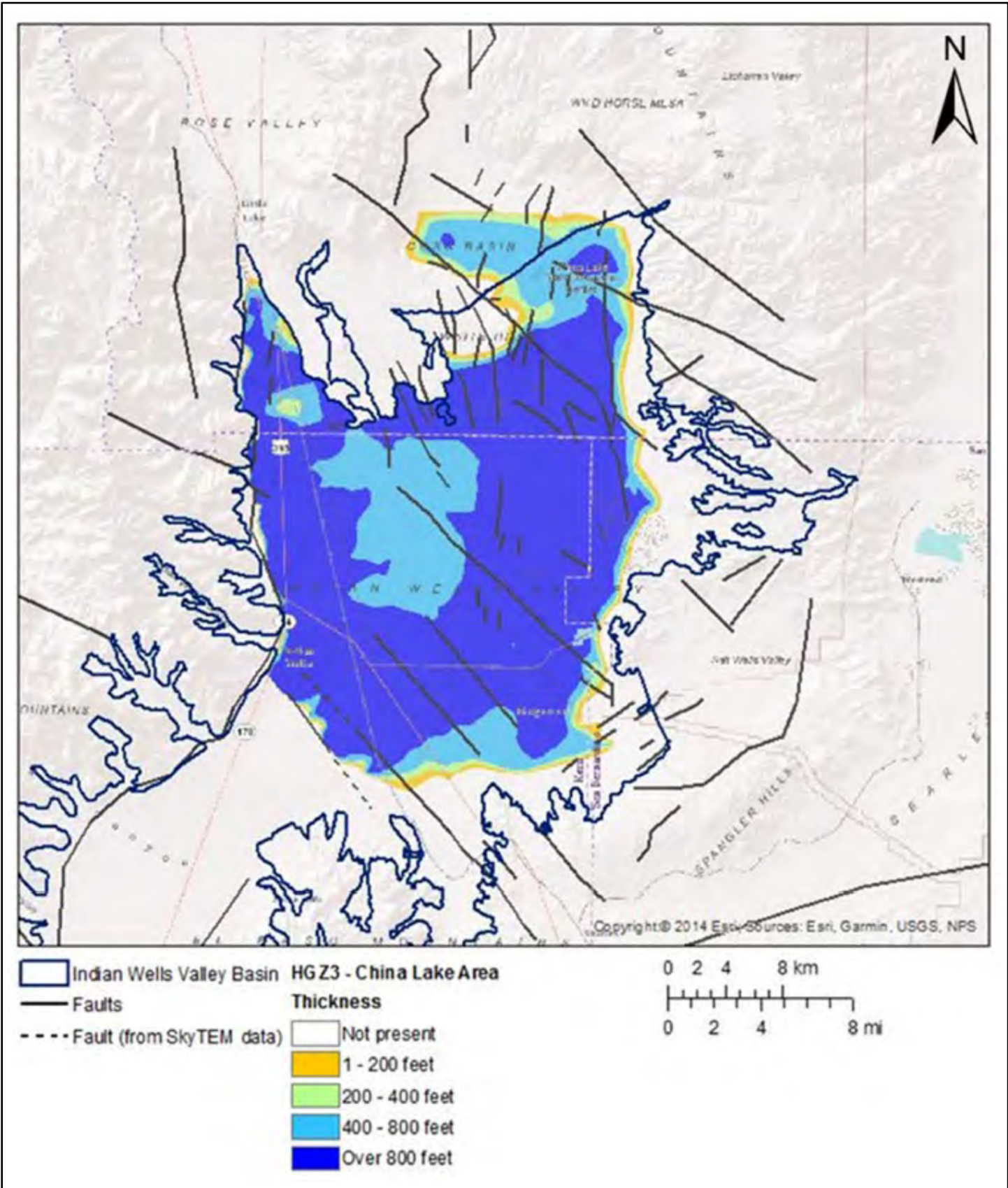
Reference: Ramboll, 2019. Hydrogeologic Conceptual Framework – Indian Wells Valley. Figure 6.6 – Thickness of the HGZ 2 in the China Lake area of the Indian Wells Valley Groundwater Basin, dated June.

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Indian Wells Valley
HGZ 2 Extent And Thickness

Date: 9/18/2023

Figure 31



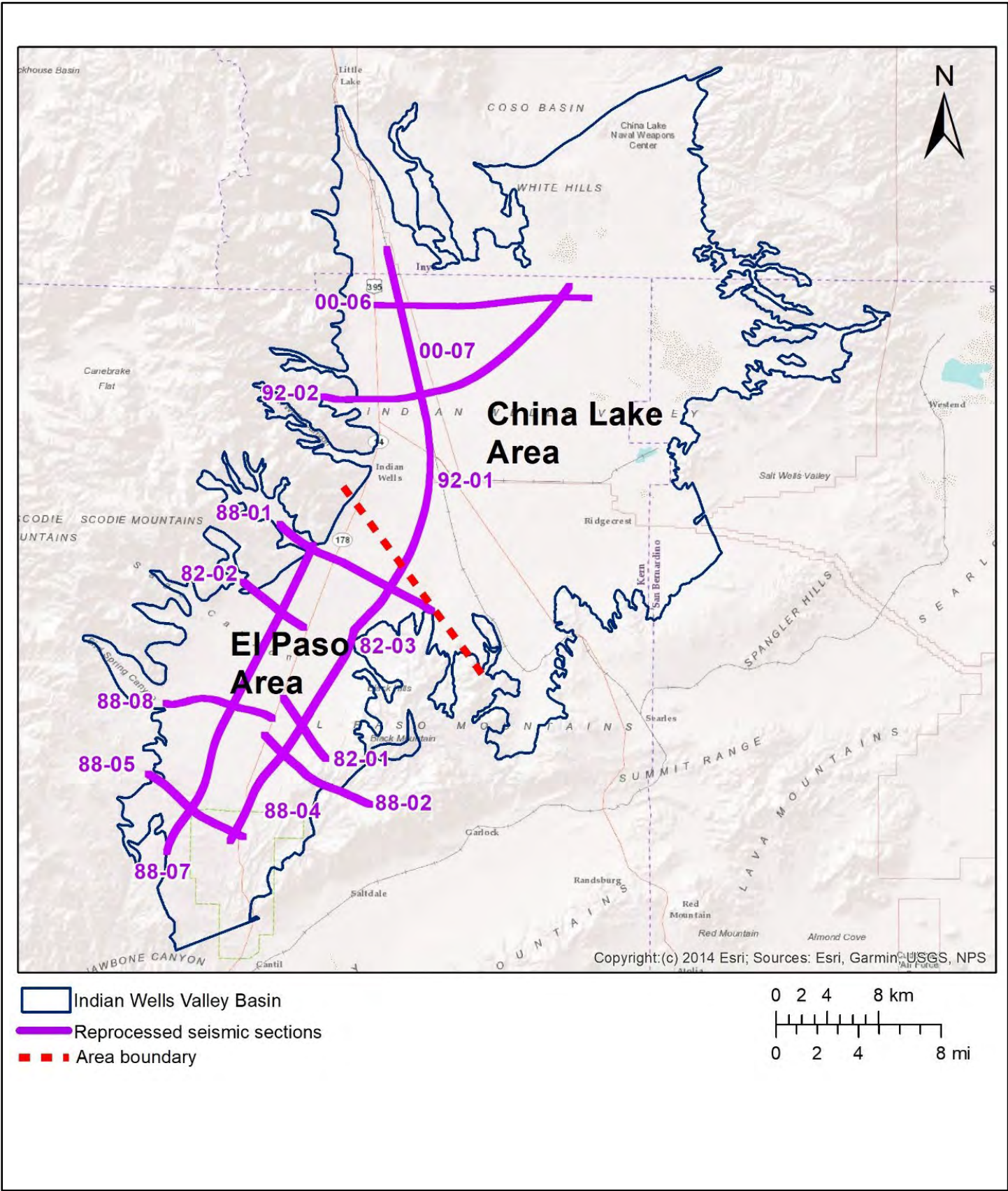
Reference: Ramboll, 2019. Hydrogeologic Conceptual Framework – Indian Wells Valley. Figure 6.7 – Thickness of the HGZ 3 in the China Lake area of the Indian Wells Valley Groundwater Basin, dated June.

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Technical Working Group

Indian Wells Valley
HGZ 3 Extent And Thickness

Date: 9/18/2023

Figure 32

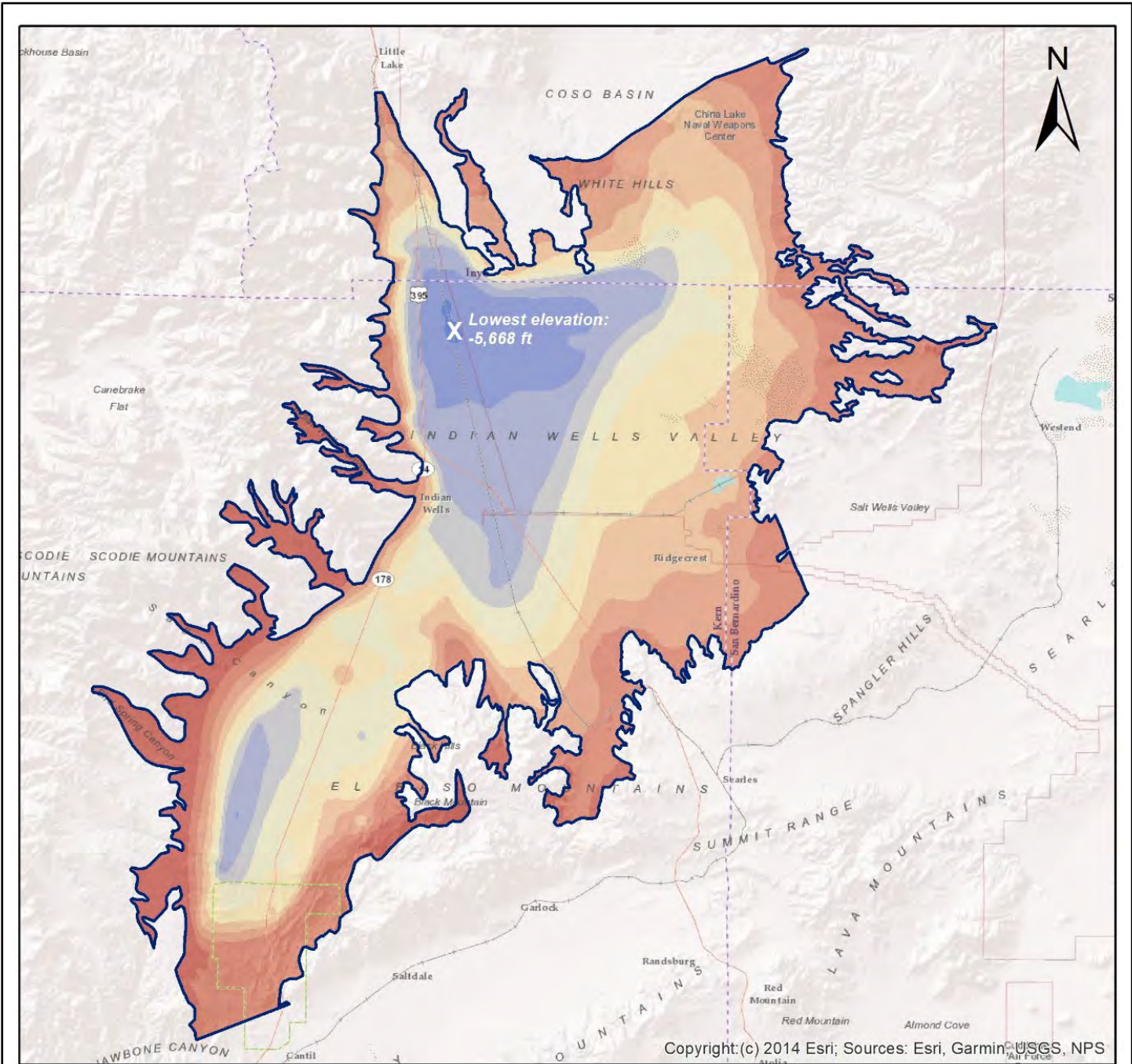


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley.
 Prepared for Indian Wells Valley Water District. September 2022,
 Revised June 2023 and February 2024.

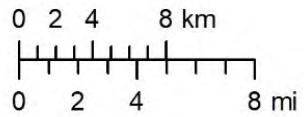
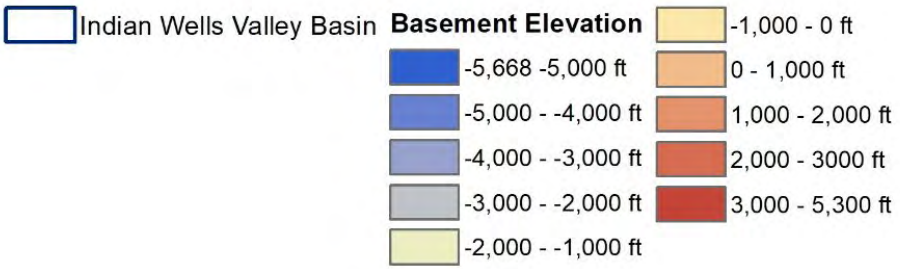
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**Location of the Reprocessed Seismic
 Lines in the Indian Wells Valley Basin**

Date: 9/18/2023		Figure 33
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Copyright: (c) 2014 Esri; Sources: Esri, Garmin, USGS, NPS

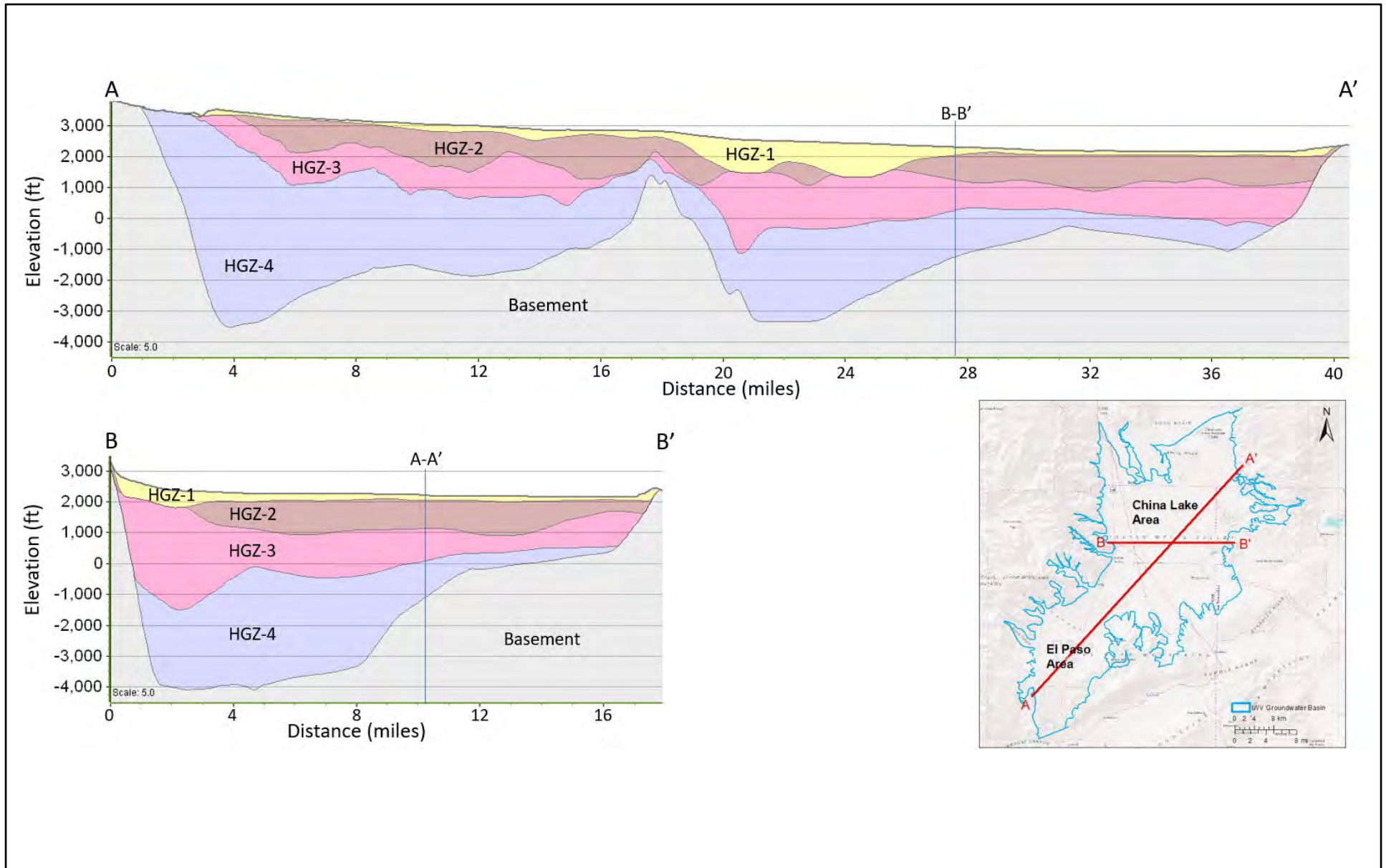


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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**Elevation of the Basement
 in the Indian Wells Valley Basin**

Date: 9/18/2023

Figure 34

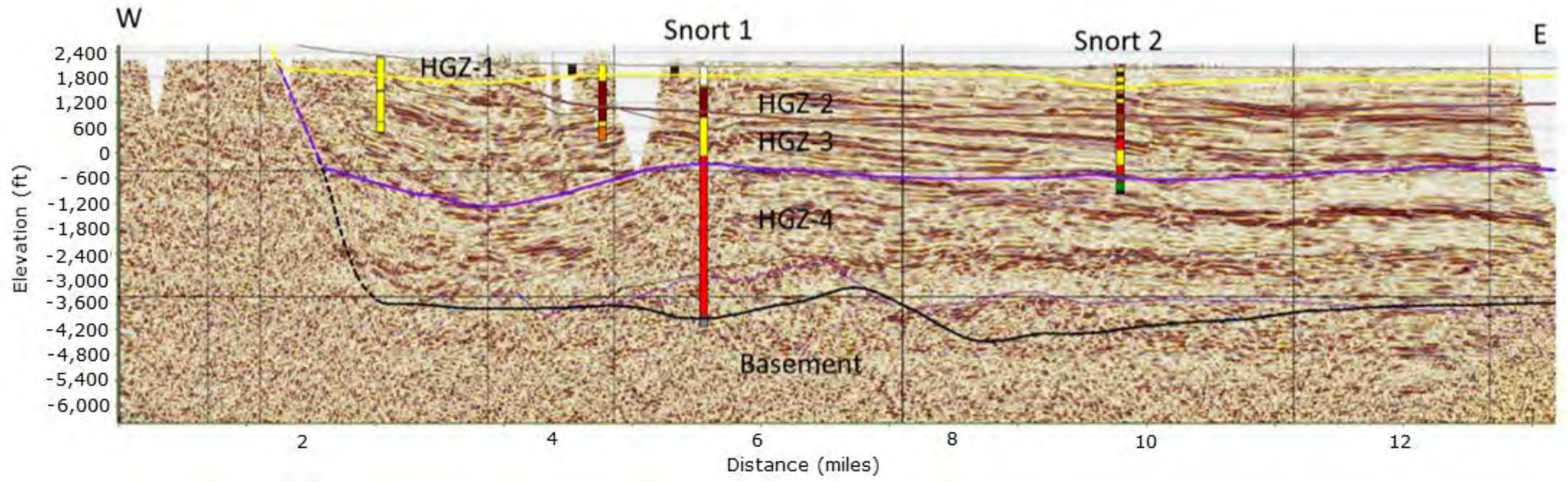


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

Indian Wells Valley
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 Indian Wells Valley
**Cross-Sections Showing the Updated HCF,
 Including the Four HGZs, & Location Map**

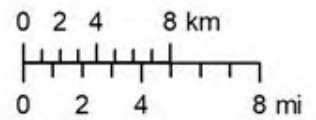
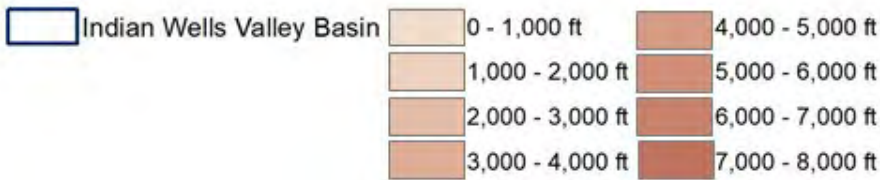
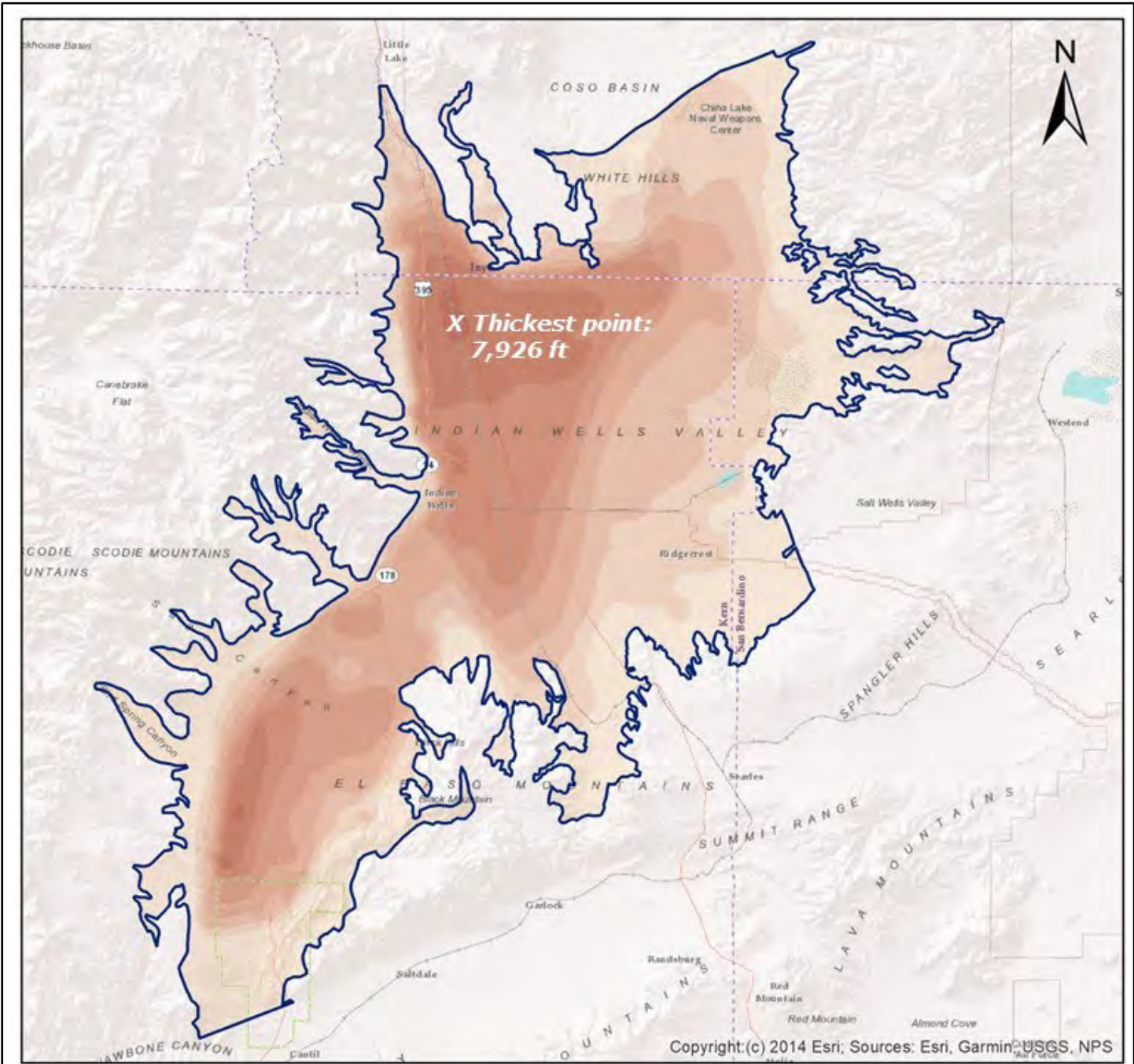
Date: 9/18/2023

Figure 35



Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

Indian Wells Valley Technical Working Group Indian Wells Valley		
Seismic Section 92-02, as Interpreted in the Updated HCF		
Date: 9/18/2023		Figure 36

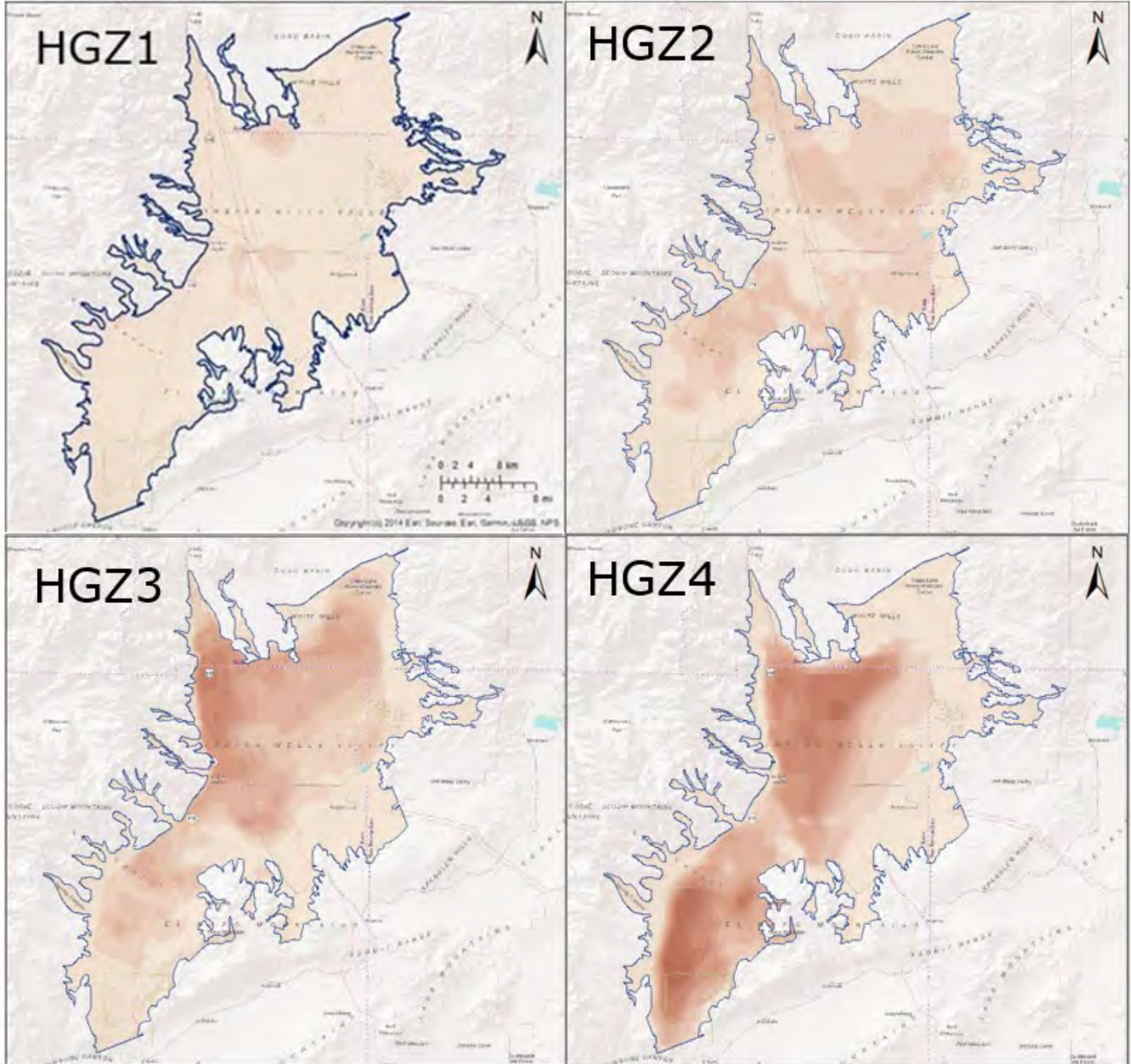


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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Indian Wells Valley
**Total Thickness of the Sediments
in the Indian Wells Valley Basin**

Date: 9/18/2023

Figure 37



Sediment Thickness

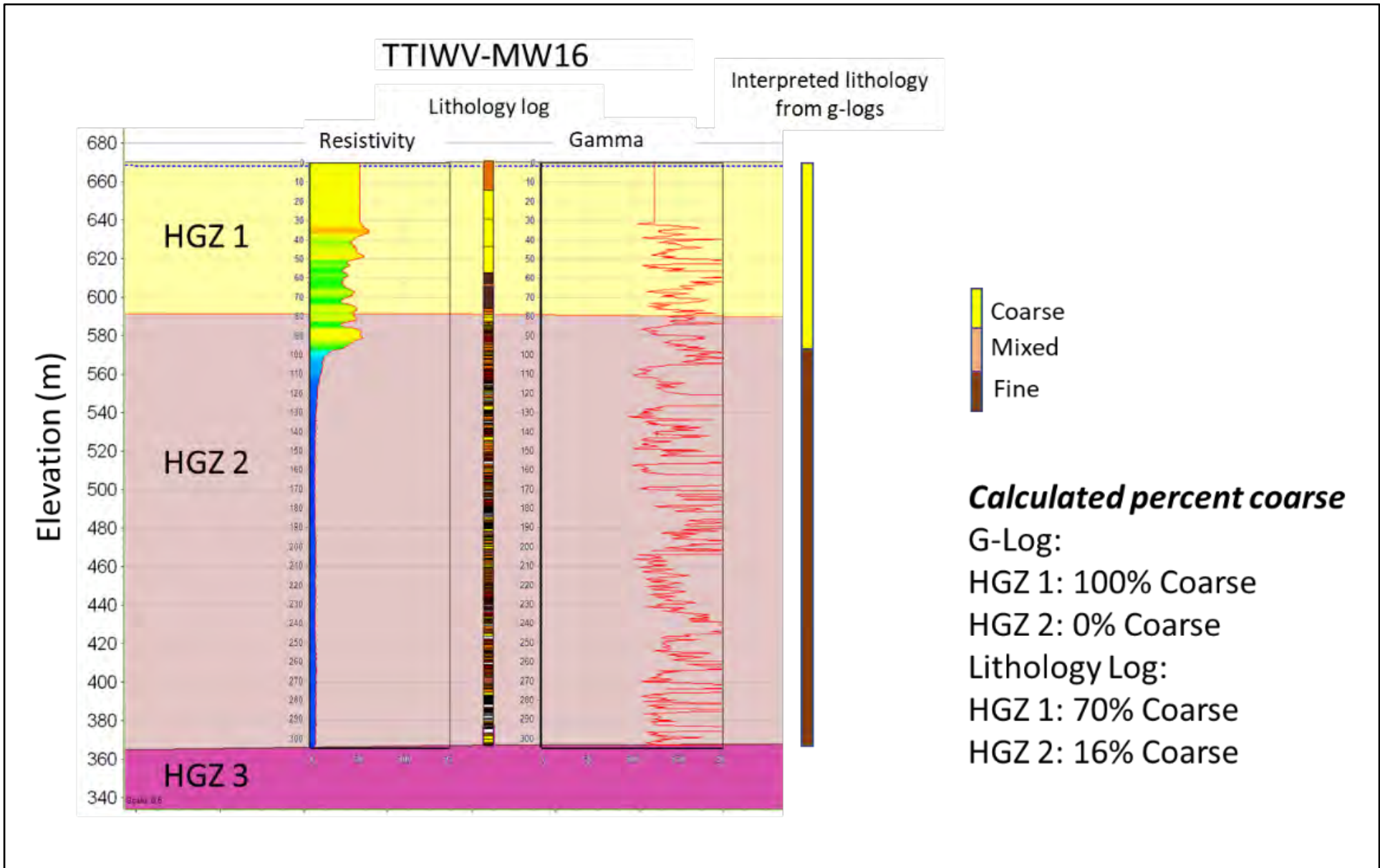


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Indian Wells Valley
**Thickness of Each HGZ
 in the Updated HCF**

Date: 9/18/2023

Figure 38



Note that the lithology log is more detailed and does not correspond fully with the resistivity and gamma log. Thus, the interpreted boundary between coarse and fine materials based on the geophysical log, shown on the right side of the figure, is lower than the interpretation from the lithology log.

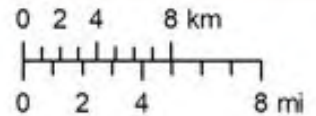
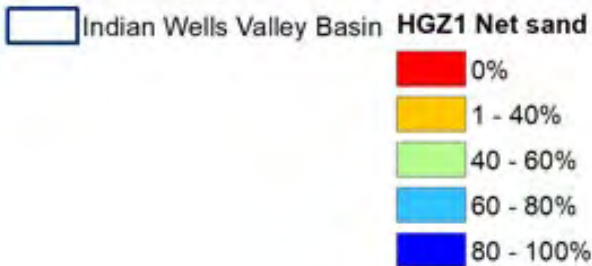
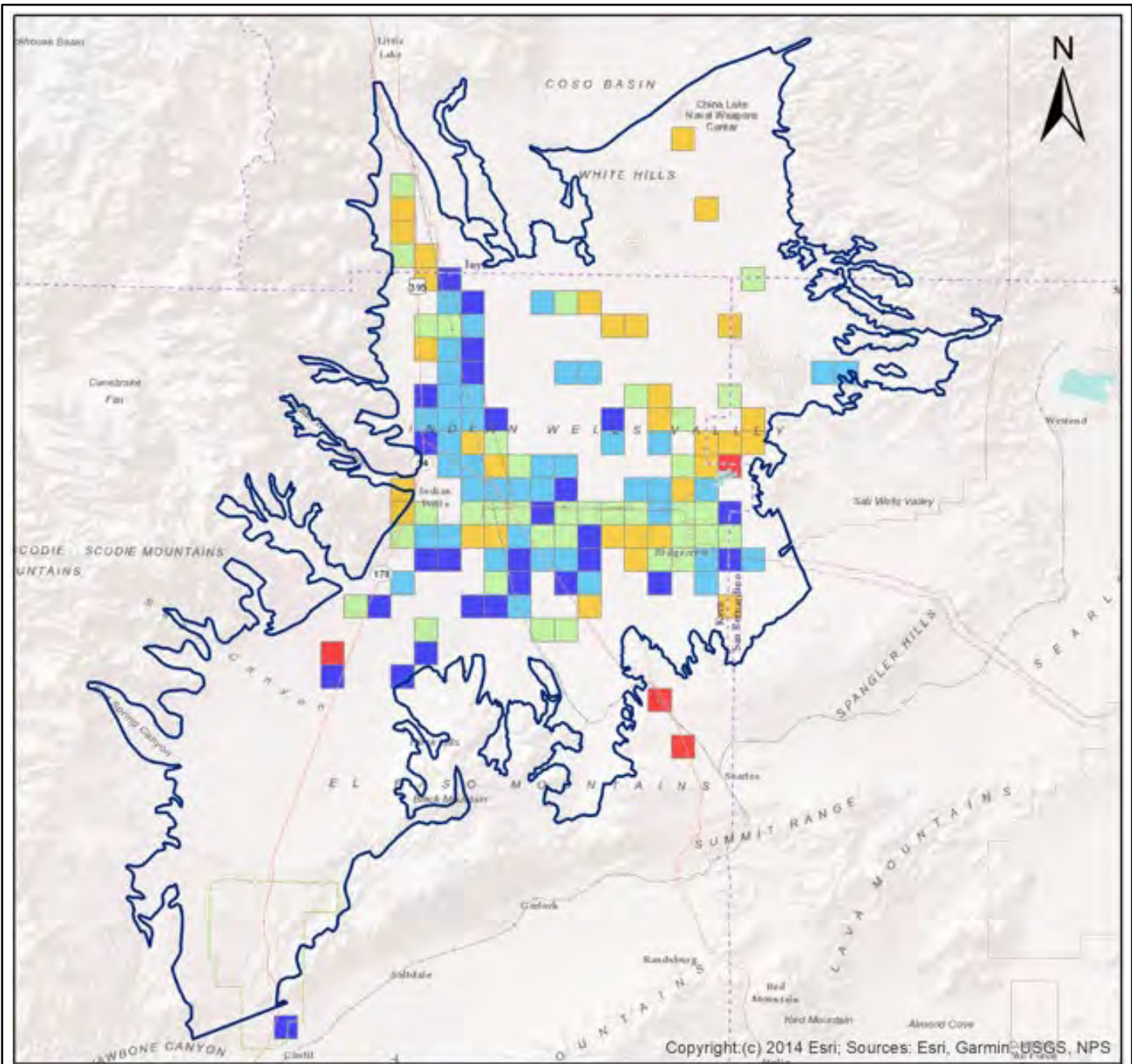
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Indian Wells Valley

**Example Showing How the Net Sand
 (Coarse Materials) was Interpreted**

Date: 9/18/2023

Figure 39



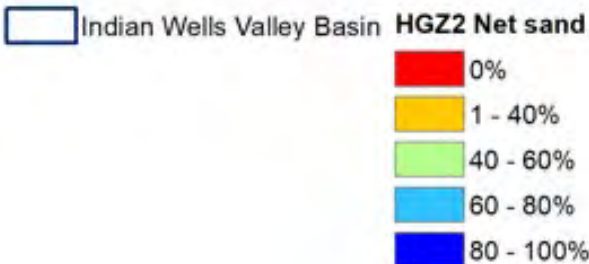
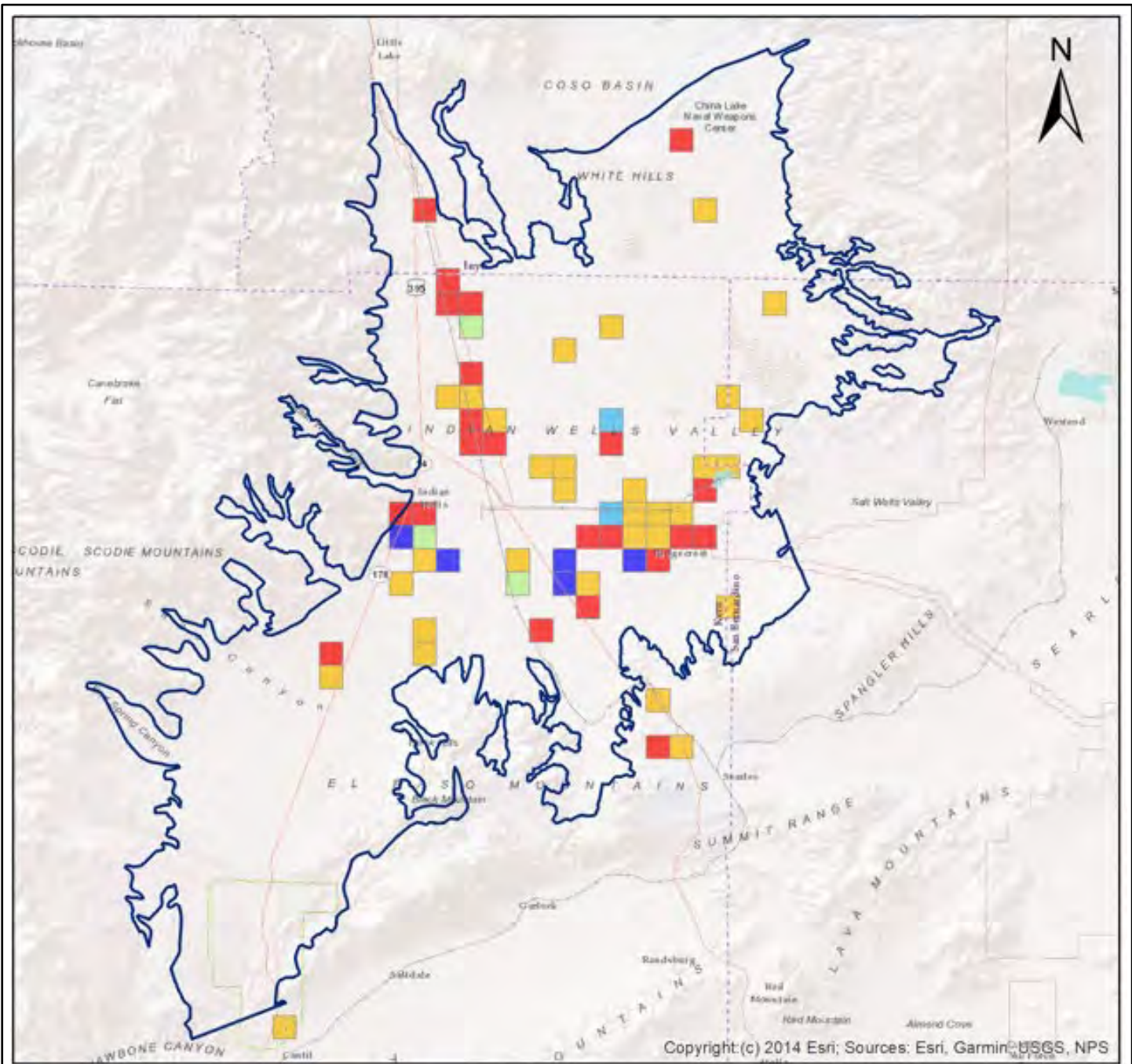
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Indian Wells Valley

**Net Sand for HGZ1, Represented
 in One-Mile Square Grids**

Date: 9/18/2023

Figure 40



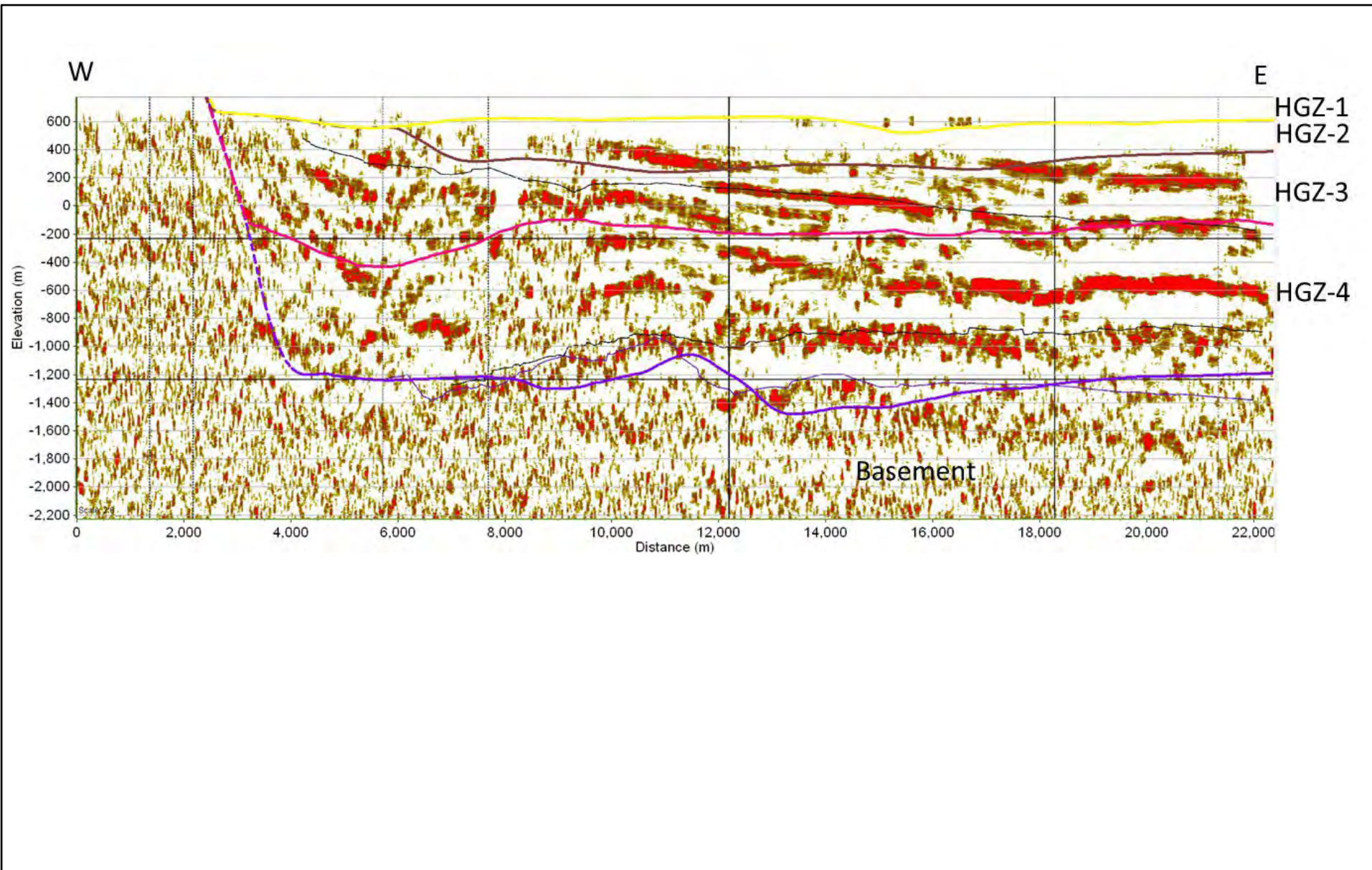
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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**Net Sand for HG22, Represented
 in One-Mile Square Grids**

Date: 9/18/2023

Figure 41



On this line, HGZ 3 contains of 19% net sand, and HGZ 4 contains 11% net sand.

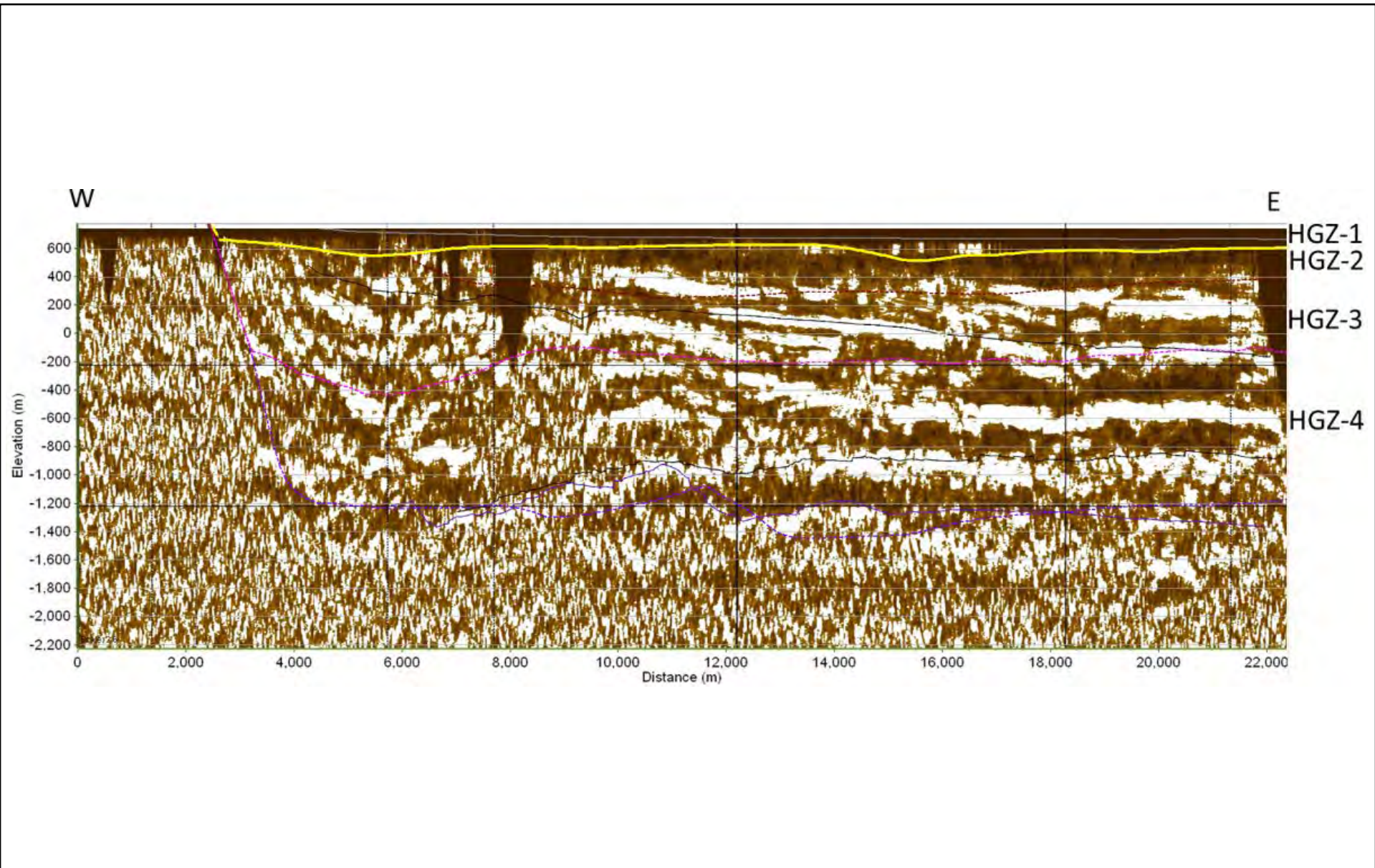
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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**Seismic Line 92-02 Showing the
 Interpreted Net Sand (Dark Red Colors)**

Date: 9/18/2023

Figure 42



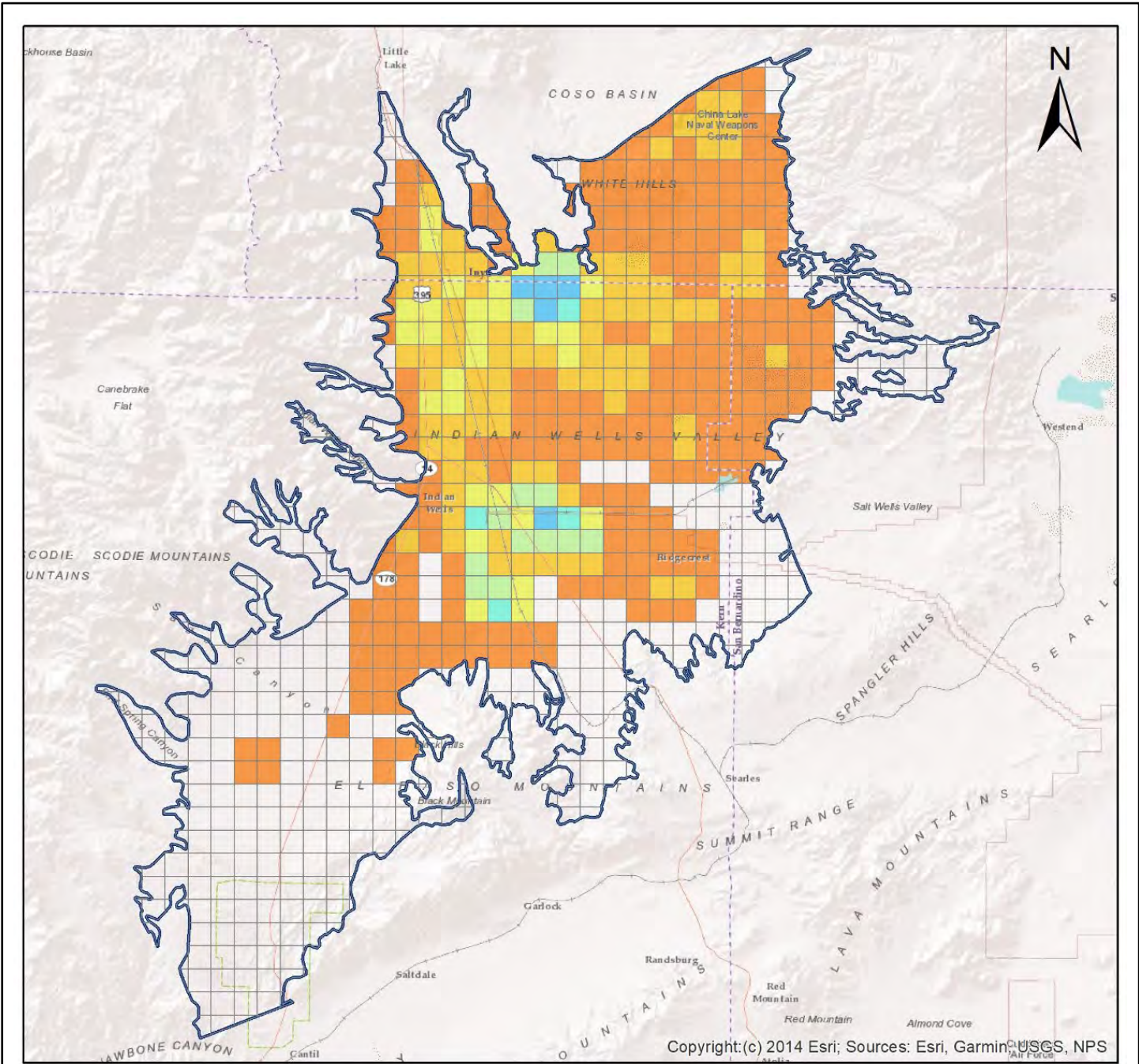
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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**Seismic Line 92-02 Showing the
 Interpreted Net Clay (Dark Brown Colors)**

Date: 9/18/2023

Figure 43

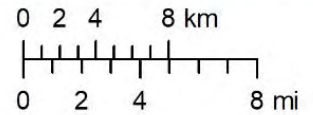


Copyright: (c) 2014 Esri; Sources: Esri, Garmin, USGS, NPS

Indian Wells Valley Basin

Storage HGZ1

	None		30,000 - 40,000 af
	0 - 10,000 af		40,000 - 50,000 af
	10,000 - 20,000 af		50,000 - 75,000 af
	20,000 - 30,000 af		75,000 - 100,000 af
			Over 100,000 af



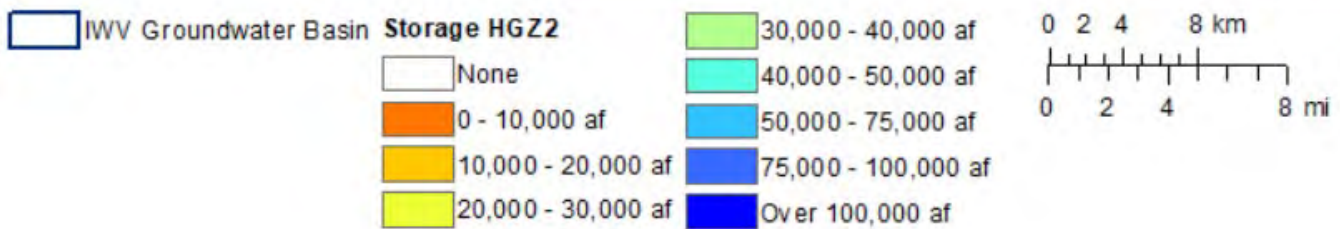
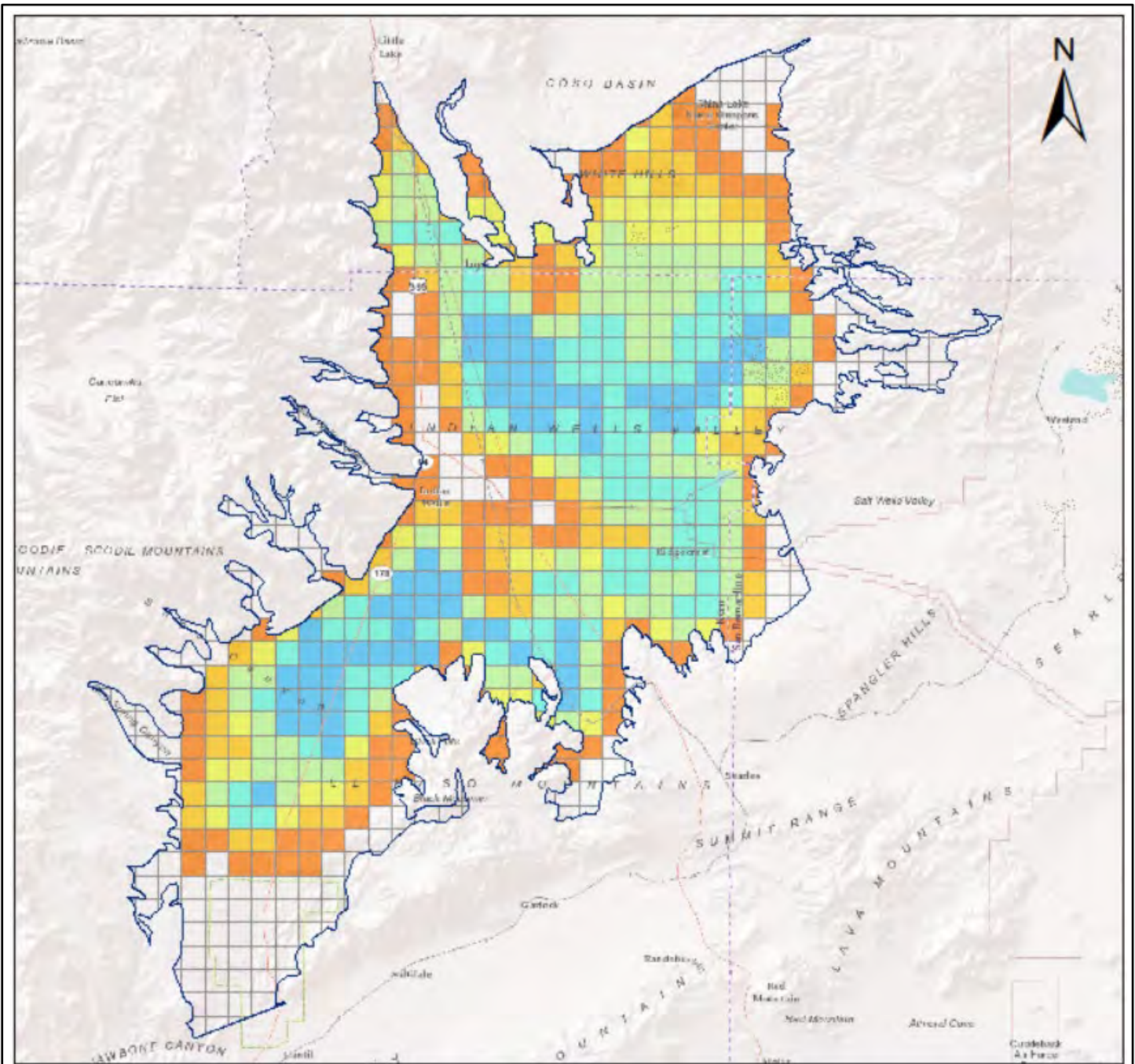
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

Indian Wells Valley
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 Indian Wells Valley

Distribution of Minimum Total Groundwater in Storage in HGZ1

Date: 9/18/2023

Figure 44



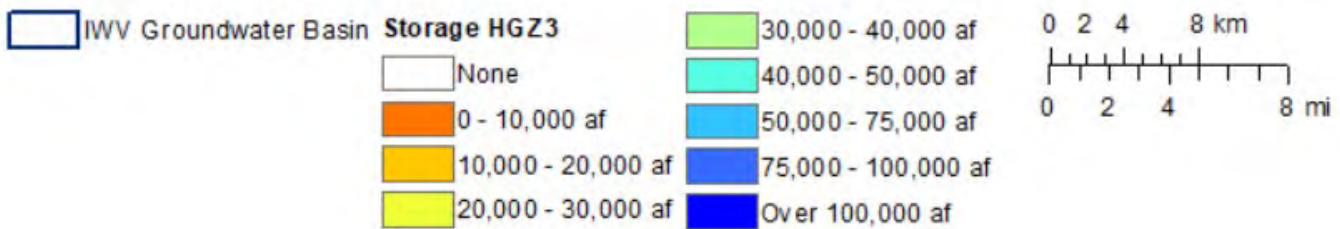
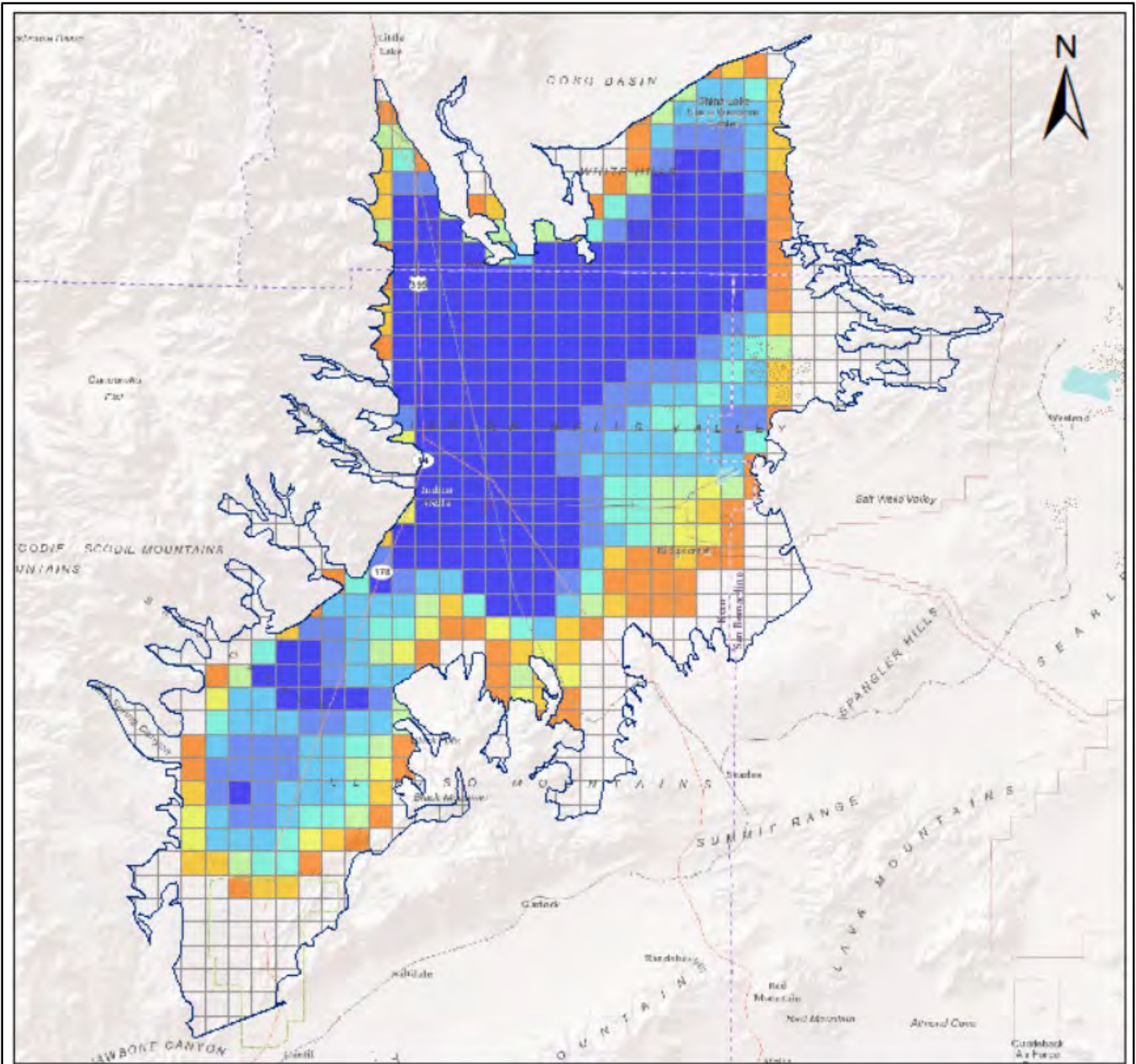
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Indian Wells Valley

**Distribution of Minimum Total
 Groundwater in Storage in HGZ2**

Date: 9/18/2023

Figure 45

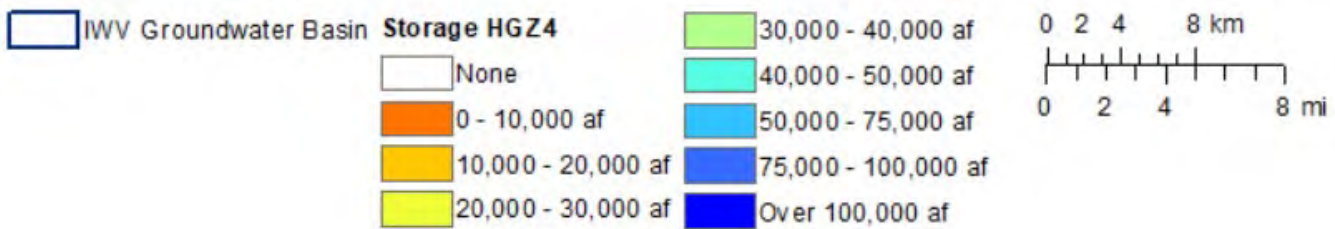
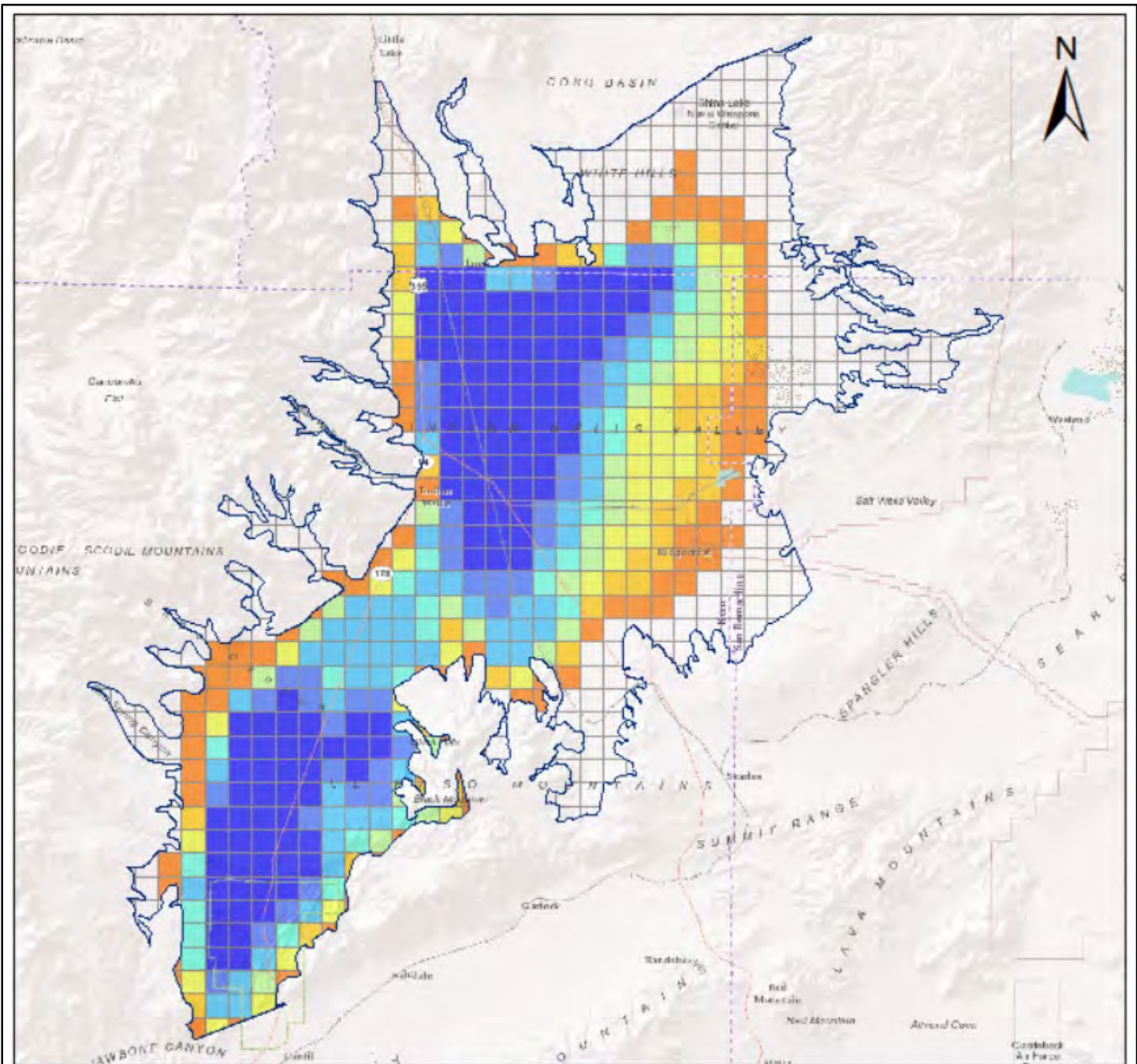


Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Indian Wells Valley

**Distribution of Minimum Total
 Groundwater in Storage in HGZ3**

Date: 9/18/2023		Figure 46
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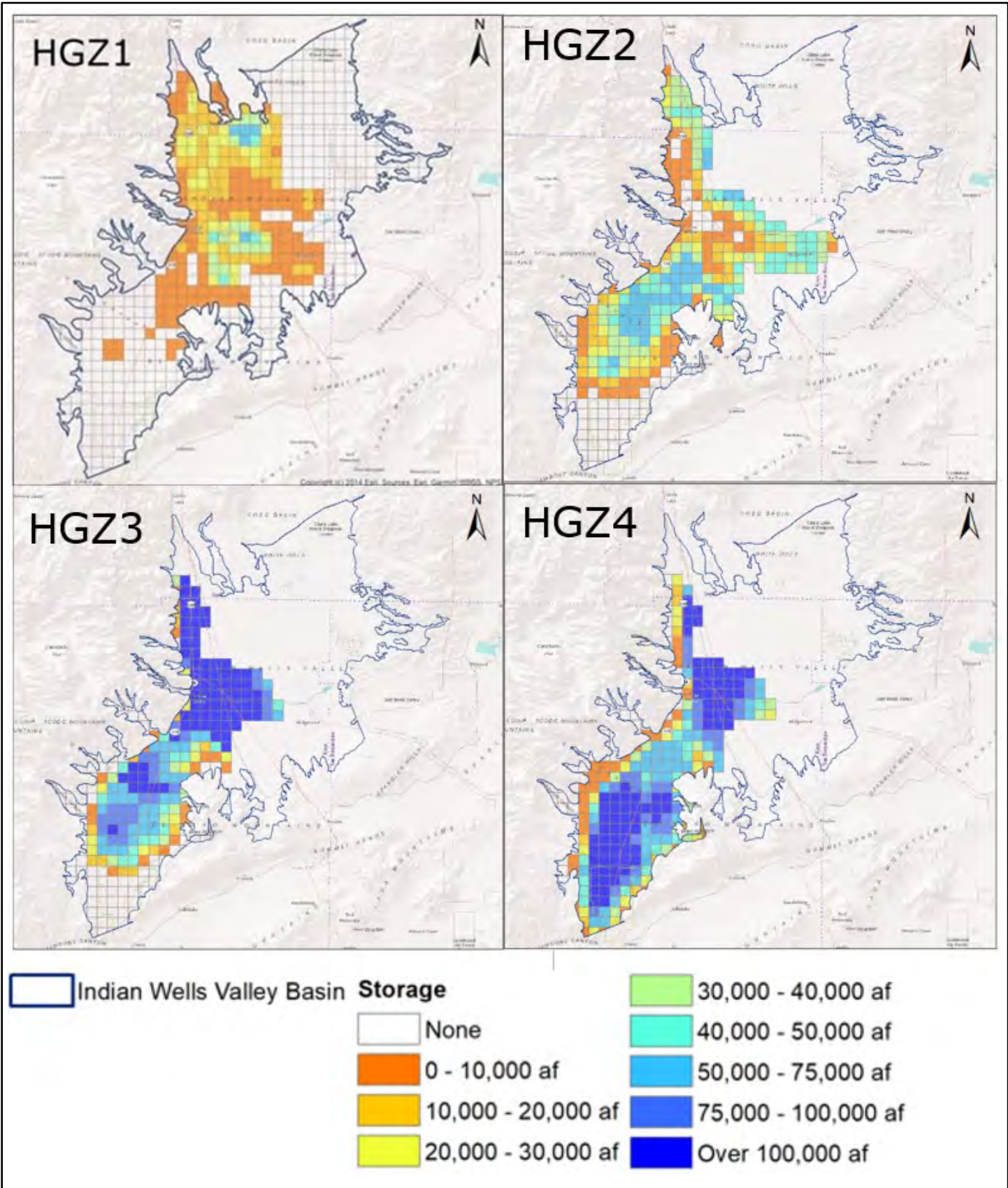
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

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 Technical Working Group
 Indian Wells Valley

**Distribution of Minimum Total
 Groundwater in Storage in HGZ4**

Date: 9/18/2023

Figure 47



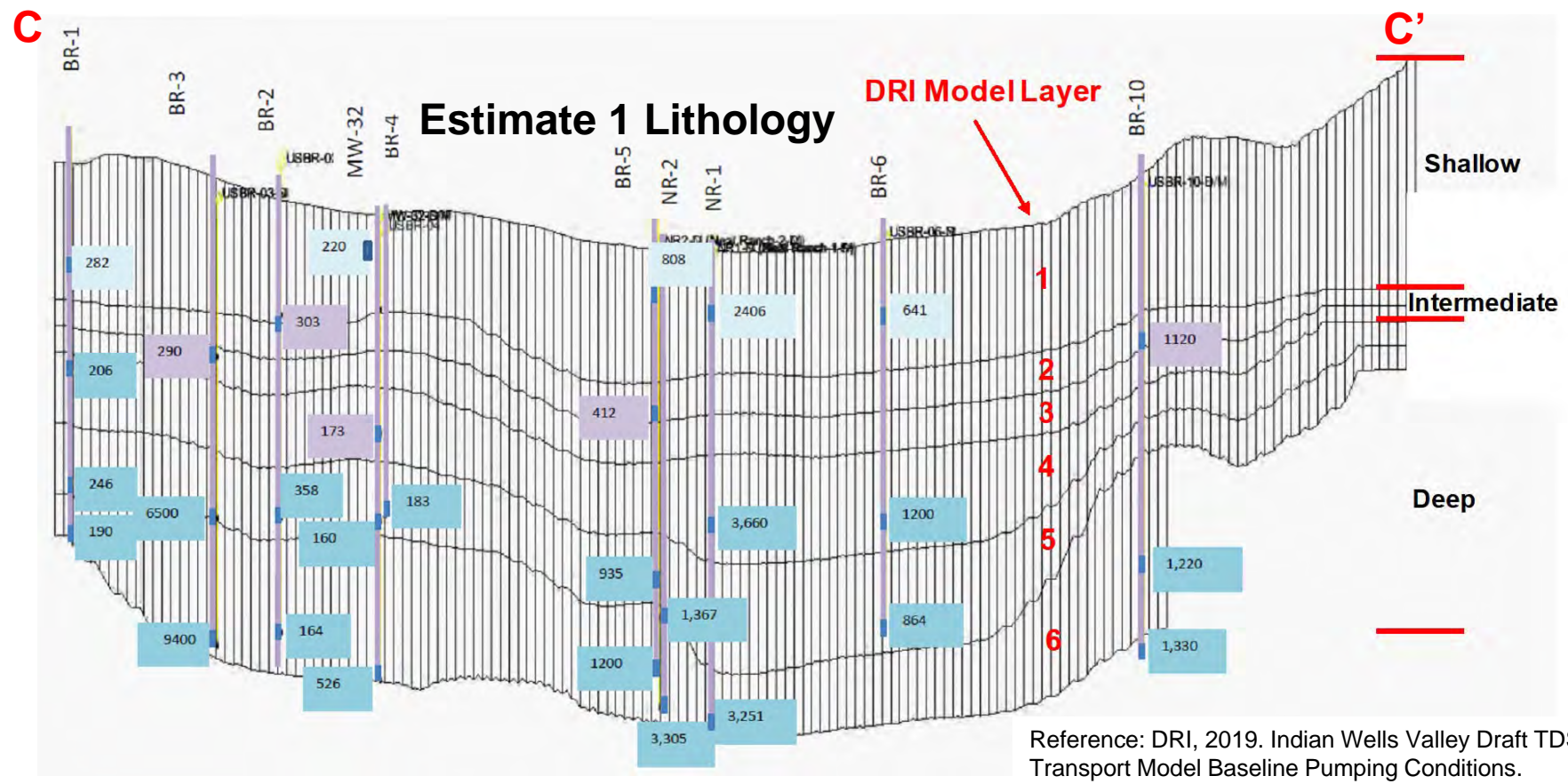
Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley.
 Prepared for Indian Wells Valley Water District. September 2022,
 Revised June 2023 and February 2024.

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**Distribution of Total Groundwater
 in Storage Under 1,000 mg/L TDS**

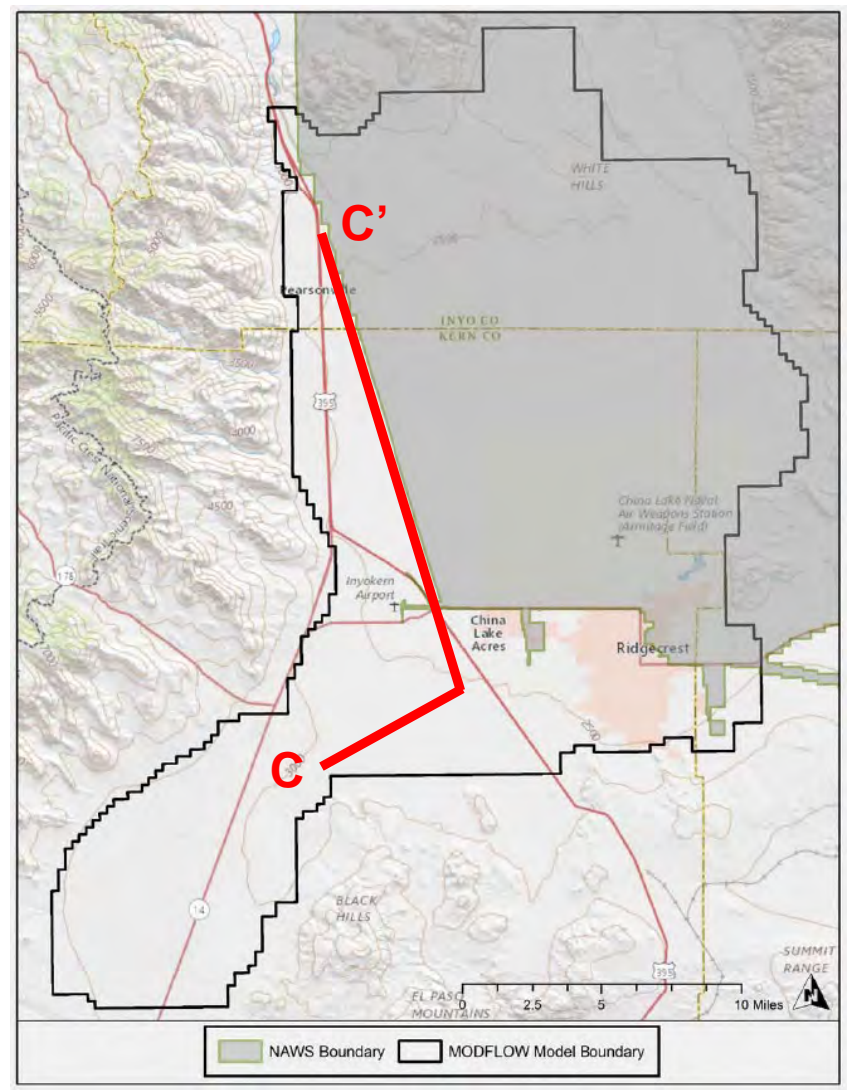
Date: 9/18/2023

Figure 48



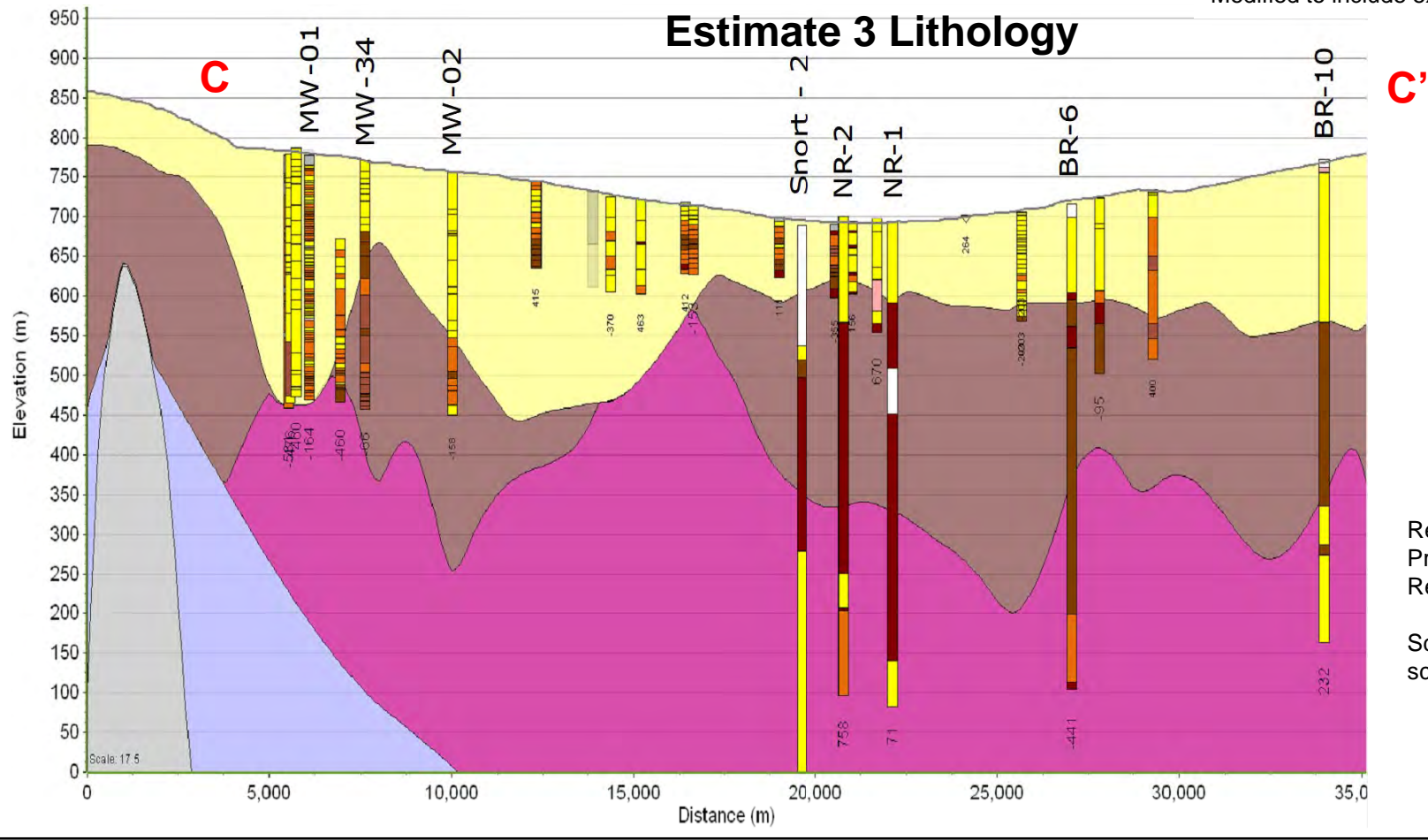
Reference: DRI, 2019. Indian Wells Valley Draft TDS Transport Model Baseline Pumping Conditions. Presentation to IWV TAC. Slide 7, dated February 7.

Modified to include explanatory labels.



Reference: Stetson Engineers. (2017). Indian Wells Valley Model Review of DRI 2017 Model Update. Figure 1. December 7.

Modified to show cross-section location.



Reference: Ramboll, 2024. Storage Estimate – Indian Wells Valley. Prepared for Indian Wells Valley Water District. September 2022, Revised June 2023 and February 2024.

Scales modified to approximately match horizontal and vertical scales of DRI cross-section C-C'.

Indian Wells Valley
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Indian Wells Valley
**Comparison of Estimate 1 Lithology
To Estimate 3 Lithology**

Date: 9/18/2023

Figure 49